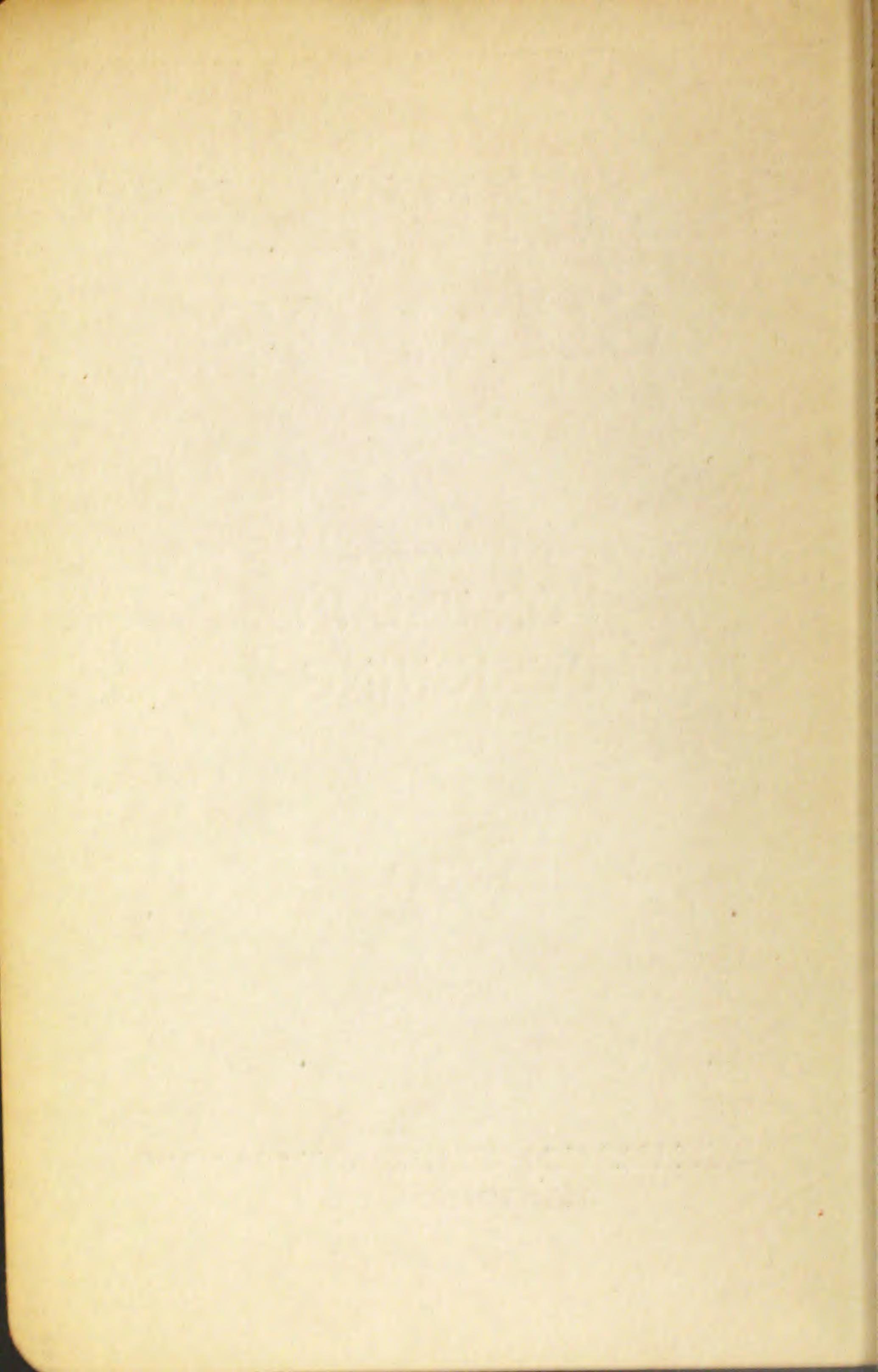
# BETHLEHEM SHAPES

ARRANGED
FOR USE IN
DESIGNIG

1930

BETHLEHEN STEEL COMPANY
BETHLEHEN, PA.



### BETHLEHEM SHAPES

ARRANGED IN THE ORDER OF
DEPTHS AND STRENGTHS
FOR CONVENIENT USE
IN
DESIGNING

Catalogue No. S-35

BETHLEHEM STEEL EXPORT
CORPORATION
437 ST. JAMES STREET
MONTREAL

BETHLEHEM STEEL COMPANY
GENERAL OFFICES: BETHLEHEM, PA.

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#### INTRODUCTION.

This booklet provides information relating to Bethlehem Structural Shapes compactly arranged for use in designing, thus promoting ease of reference and ready selection of the most economical sections.

The first tables comprise the principal dimensions and properties of all Bethlehem shapes best adapted for use as beams, arranged substantially in the order of their flexural strengths.

Following this are tables of unit stresses to be used in designing centrally loaded columns for various usual ratios of length to least radius of gyration, these being based on the A. I. S. C. formula.

The last table herein shows the most economical Bethlehem shape when used as a beam in flexure corresponding to any required section modulus from 6.43 for the smallest 6-inch joist to 1103.6 for the largest 36-inch girder.

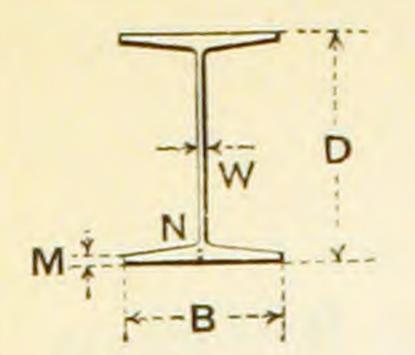
These tables provide all the usual information necessary in beam and column design.

In computing the weights and properties of all sections the fillets have been included.

The dimensions, areas, and weights presented herein are theoretical and subject to the usual variations.

Other information relating to Bethlehem Structural Shapes is shown in the following catalogues: S-27, "BETHLEHEM STRUCTURAL SHAPES"; S-34, Supplement thereto; and S-28, "BETHLEHEM STEEL JOISTS AND STANOHIONS, LTC."

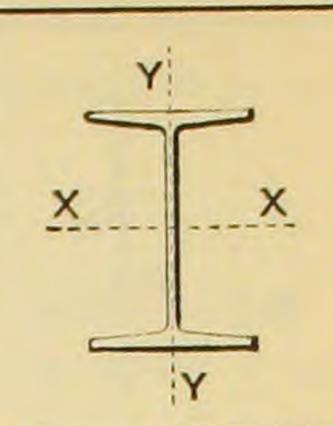
Standard structural shapes, including American Standard I Beams and Channels, Ship Channels, Equal and Unequal Angles, Bulb Angles, Z-Bars, and Car Building Shapes are shown in catalogue S-30, entitled "STANDARD STRUCTURAL SHAPES, ETC."



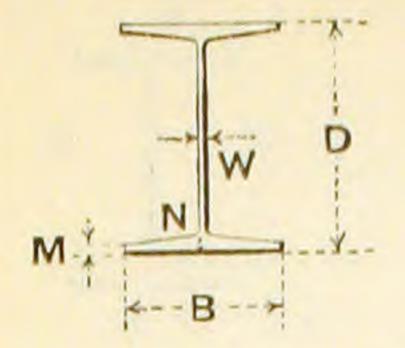
### BETHLEHEM SHAPES

		Nominal	Width		CKNES	100	Area	AX	IS X->	(
Section Number	Weight per Foot, Pounds	Depth of Beam, Inches	of Flange, Inches	Web	Flar	ige	of Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modulus Inches <sup>3</sup>
		D	В	w	М	N		1	r	S
						0.005	00.10	20.000	15 16	1103.6
G36 x	300.0	36.720	16.655	.945	1.353	2.007	88.12	20,202	15.16	1030.8
G36 x	x 280.0	36.500	16.600	.890	1.243	1.897	82.45			949.5
G36 x	x 260.0	36.240	16.555	.845	1.113	1.767	76.50			
G36	x 250.0	36.120	16.530	.820	1.053	1.707	73.61			911.2
G36 :	x 240.0	36.000	16.500	.790	.993	1.647	70.55			872.0
G36 :	x 230.0	35.880	16.475	.765	.933	1.587	67.67	14,960	14.87	833.9
B36	x 190.0	36.520	12.111	.726	1.035	1.509	55.87	12,049	14.68	
R36	v 173 0	36.250	12.065	.680	.900	1.374	50.94	10,784	14.55	
B36	x 164.0	36.120	12.030	.645	.835	1.309	48.10	10,133	14.51	
B36	x 155.0	36.000	12.000	.615	.775	1.249	45.58	9547.4		
B36	x 147.0	35.900	11.968	.583	.725	1.199	43.23	9036.3	14.46	503.4
Can	260 0	33.630	15 890	875	1.237	1.863	76.54	14,868	13.94	884.2
Cos	X 200.0	33.440	15.850	835	1.142	1.768	72.19	13,895	13.87	831.0
(33	x 240.0	33.250	15.810	795	1.047	1.673	67.85	12,935	13.81	778.0
G33	X 230.0	33.120	15.780	765	982	1.608	64.80	12,278	13.77	741.4
(199	x 220.0	33.000	15.750	735	922	1.548	61.91	11,671	13.73	707.3
(199	x 210.0	22 880	15.715	700	.862	1.488	8 58.87	11,055	13.70	672.4
Goo	x 200.0	02.000	10.110							
D22	v 165 (	33.500	11.350	.680	.968	3 1.41	2 48.52	8835.4	13.49	527.
		33.270					7 44.65		13.38	480.4
		33.120			1		2 42.05		13.30	
		33.000					2 39.55	6967.4		
		0 32.890					7 36.83		13.28	395.
1500						1				

### BETHLEHEM SHAPES



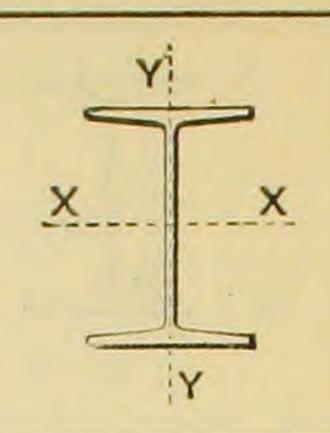
Weight	Maxi- mum Safe	NCE IN	RESISTA FOOT PO	COEFFICIENTS OF STRENGTH IN FOOT POUNDS			KIS Y-Y	Α)
	Shear	For Fiber Stress of 16,000 Lbs. per Sq. In.	per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	Section Modu- lus, Inches <sup>3</sup>	Radius of Gyra- tion, Inches	Moment of Inertia, Inches
		R'	R		<u>C</u>	S'	r'	I'_
300.0	334 800	1 472 000	1 655 000	11,770,000	12 240 000	141 4	2 66	11777
				10,990,000			3.66	177.7
				10,330,000			3.57	1081.4
					10,940,000		3.54	973.7 923.8
					10,460,000		3.52	873.5
					10,010,000		3.49	824.5
				0,000,000	10,010,000	100.1	0.10	021.0
190.0	209,300	879,800	989,800	7,039,000	7,918,000	57.0	2.48	344.9
173.0	184,000	793,300	892,500	6,347,000	7,140,000	49.9	2.43	301.1
164.0	165,600	748,100	841,600	5,985,000	6,733,000	46.5	2.41	279.4
155.0	150,300	707,200	795,600	5,658,000	6,365,000	43.3	2.39	259.9
147.0	134,500	671,200	755,100	5,370,000	6,041,000	40.7	2.37	243.3
260.0	285,900	1,179,000	1,326,000	9,432,000	10,610,000	118.3	3.50	939.8
			1,247,000		9,972,000	109.7	3.47	869.2
			1,167,000		9,337,000	101.2	3.43	799.6
			1,112,000	,		95.3	3.41	752.2
	211,000			7,545,000		90.0	3.38	708.5
200.0	193,200	896,600	1,009,000	7,173,000	8,069,000	84.6	3.36	664.6
165.	183,200	703,300	791,200	5,627,000	6,330,000	46.8	2.34	265.5
	163,700							234.9
143.	150,300	599,200	674,100					215.1
135.	133,700	563,000	633,400	4,504,000	5,067,000	35.3	2.24	198.7
125.	113,300	526,900	592,700	0 4,215,000	4,742,000	32.7	2.23	183.2



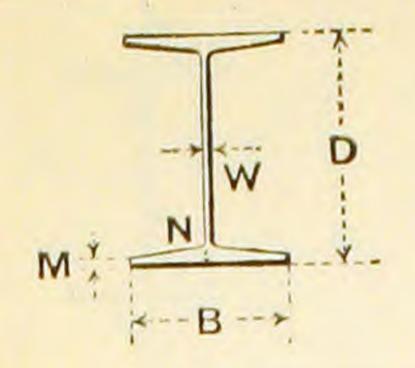
### BETHLEHEM SHAPES

Section	Weight	Nominal Depth	Width	IN	INCH	ES	Area		XIS X-	
Number	Foot, Pounds	of Beam, Inches	of Flange, Inches	Web	Fla	nge	Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion,	Section Modulus, Inches <sup>3</sup>
		D	В	W	M	N		I	Inches	s
			1 - 000	000	1.004	1.001	70.00	11 100	19.79	7190
G30 x		30.750	15,200	.880		1.801		11,423	$12.72 \\ 12.65$	$742.9 \\ 680.5$
G30 x			15.135				64.82	10,378 $9343.8$		617.8
G30 x			15.065	The state of the s	The second second		58.92 55.90	8818.0		585.5
G30 x			15.030				53.20	8343.1	The state of the s	556.2
G30 x			15.000				50.80	7895.2		528.5
G30 x	173.0	29.880	14.980			E				
B30 x	163.0	The second secon	10.680				48.00	7270.7		474.4
B30 x	149.0		10.620		2 - 2		43.93	6606.6		434.1
B30 x		The second second second	10.570	THE PARTY OF THE PARTY.			40.40	6026.7		398.5
B30 x			10.530				37.82	5622.7		373.4
B30 x		Contract of the Contract of th	10.500			The state of the s	35.65	5269.7 $4942.9$		351.3 330.8
B30 x			10.480				33.80	4687.7		314.8
B30 x	110.0	29.780	10.470	.520	.030	1.045	32.45	4007.7		314.0
G28 x	186.0	28.310	14.305	.730			54.73	7604.0		537.2
G28 x	175.0		14.285				51.45	7026.0		499.7
G28 x	165.0		14.250	501 miles		The second second second	48.75	6624.6		473.2
G28 x			14.210	5			45.93	6218.6		446.1
G28 x	145.0	27.750	14.160	.585	.675	1.241	42.69	5772.3	11.00	416.0
B28 x	133.0	28.590	10.160	.630	.894	1.291	39.09	5204.0		364.0
	119.0	28.380	10.095	.565		The second second second	35.11	4647.4		327.5
B28 x	112.0	28.250	10.065	.535		The second second second	32.95	4328.0		306.4
B28 x	104.0	28.120	10.030	.500			30.66	4003.3		284.7
B28 x	97.0	28.000	10.000				28.61	3711.5		265.1
B28 x	91.0	27.880					26.86	3441.1		
B28 x	85.0	27.690	9.980	.450	.444	.841	24.96	3075.2	11.10	222.1
G26 x	160.0	26.120	13.790	.670	.845	1.392	47.25	5629.4	A CONTRACTOR OF THE PARTY OF TH	431.0
G26 x		26.000	13.750	.630		The state of the s	44.55	5289.8		406.9
G26 x	144.0		13.730				42.38	4983.4		385.1
G26 x	138.0	25.810	13.700	.580	.690	1.237	40.65	4779.9	10.84	370.4
B26 v	98.0	26.120	9.530	.500	.665	1.042	28.69	3231.2	10.61	247.4
	91.0	26.000	9.500	.470			26.76	2993.1		230.2
	85.5	25.880	19 19 19 19 19 X	.450	400000000000000000000000000000000000000		25.11	2772.5	The same of the same of	214.3
The second second	81.0	25.780	9.470	.440	.495	.872	23.90	2600.1	10.43	201.7

#### BETHLEHEM SHAPES



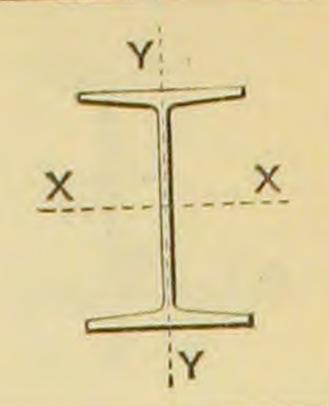
A	XIS Y-Y		COEFFICII STRENG FOOT P	TH IN	MOMEN RESISTA FOOT P	NCE IN	Maxi- mum	
Moment of Inertia Inches <sup>4</sup>	Radius of Gyra- tion Inches	Section Modu- lus Inches³	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Safe Shear on Web, in Pounds	Weight per Foot, Pounds
	-							
799.2	3.36	105.2	8,915,000	7,925,000	1,114,000		277,300	
716.1	3.32	94.6	8,166,000	7,259,000	1,021,000		245,000	220.0
634.2	3.28		7,413,000				210,900	
592.7	3.26		7,026,000		1		194,100	
555.1	3.23		6,674,000			741,600	179,800	
519.1	3.20	69.3	6,342,000	5,637,000	792,700	704,600	170,200	173.0
239.8	2.24	44.9	5,693,000	5,061,000	711,700	632,600	204,100	163.0
214.5	2.21	40.4		4,630,000		578,800	175,400	149.0
192.6	2.18	36.4		4,250,000	the second	531,300	152,100	137.0
177.6	2.17	33.7	4,480,000	3,982,000	560,000	497,800	134,000	129.0
164.3	2.15	31.3	4,216,000	3,747,000	527,000	468,400	120,700	121.0
151.8	2.12	29.0		3,529,000		441,100	111,900	
141.8	2.09	27.1	3,778,000	3,358,000	472,200	419,800	107,500	110.0
539.7	3.14	75.5	6,446,000	5,730,000	805,800	716,300	199,400	
491.1	3.09	68.8	5,997,000	5,330,000		666,300	189,500	
458.3	3.07	64.3		5,047,000		630,900	173,900	1 2 2 2 2
425.4	3.04			4,758,000		The second second	156,400	The state of the s
389.8	3.02	55.1	4,992,000	4,438,000	624,000	554,700	135,100	145.0
175.3	2.12	34.5	4,369,000	3,883,000	546,100	485,400	155,100	133.0
153.7	2.09	30.5	3,930,000	3,493,000	491,300	436,700	126,900	The second secon
141.2		28.1		3,268,000			114,100	A STATE OF THE PARTY OF THE PAR
128.7				3,037,000				
117.4		23.5		2,828,000		353,500	87,900	
106.7				2,633,000		329,100	80,100	1 2 2 2
91.0	1.91	18.2	2,665,000	2,369,000	333,200	296,200	79,600	85.0
432.8			V	4,598,000			168,200	The second second second
402.8				4,340,000	the second secon			
375.0				4,108,000		513,500	143,200	
357.4	2.97	52.2	4,445,000	3,951,000	555,600	493,900	131,300	138.0
110.6	1.96	23.2	2,969,000	2,639,000	371,100	329,900	99,600	
100.9	1.94	21.2	2,763,000	2,456,000	345,400	307,000		
91.7		The second second		2,285,000		285,700		
84.3	1.88	17.8	2,421,000	0 2,152,000	302,600	269,000	76,800	81.0
							1	1



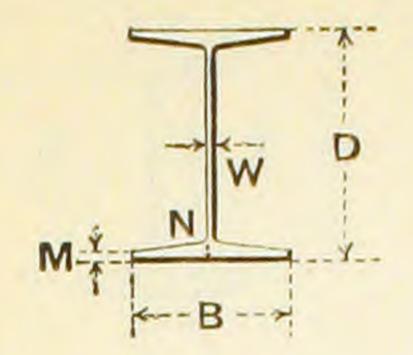
### BETHLEHEM SHAPES

1		Nominal	VA/: dth		INCHE		Area	A	XIS X-	×
Section Number	Weight per Foot, Pounds	Depth of Beam, Inches	Width of Flange, Inches	Web	Flar	nge	of Section, Square Inches	Moment of Inertia, Inches <sup>4</sup>	Radius of Gyra- tion, Inches	Section Modulus Inches <sup>3</sup>
		D	В	w	M	N		I	r	S
GQ.	1400	04 190	13.280	.640	840	1.367	43.68	4478.0	10.13	371.3
	x 148.0							4201.3	10.11	350.1
G24a : G24a :	x 140.0 x 132.0	$\begin{vmatrix} 24.000 \\ 23.880 \end{vmatrix}$	$13.240 \\ 13.210$	.600	.720	1.247	38.82			
		24.120					37.79	3867.1		320.7
G24	X 128.0	24.120					35.36	3607.8		300.6
G24	X 120.0	24.000	12.240	500			33.18	3363.3		281.7
G24 G24	x 113.0 x 107.0	23.880	12.195	A TOTAL CONTRACTOR OF THE PARTY			31.60	3173.1		266.9
					750	1.134	30.88	2997.3	9.85	248.8
		24.090 24.000	9.750				29.40	2841.3	9.83	236.8
		23.910	9.730				28.05	2692.7	9.80	225.2
R240	v 90 !	24.120	9.515	.475	.630	1.007	26.47	2588.2	9.89	214.6
B24a	x 84.	24.000	9.500	.460	.570	.947	24.97	2405.7	9.82	200.5
B24	v 79	5 24.090	9.035	.430	.565	.924	23.35	2266.7	9.85	188.2
		5 24.000			.520	.879	21.70	2108.8	9.86	175.7
The state of the s		0 23.880			.460	.819	20.62	1954.1	9.74	163.7
									0.40	210.0
G22	x 132.	0 22.380	13.095	.575	.769	1.291	38.96	3501.2	9.48	312.9
G22	x 124.	$0 \mid 22.250$	13.065	.545	.704	1.226	36.59	3261.7	9.44	293.2
		0 22.120					34.12			273.2 $254.9$
G22	x 108.	0 22.000	13.000	.480			31.89			236.8
G22	x 101.	0 21.880	12.970	.450	.519	1.041	29.68	2590.4	9.34	200.0
B220	v 96	5 22.250	9.320	.525	.754	1.120	28.38	2373.7	9.15	
		0 22.120				1.055	26.28	2188.6		197.9
		0 22.000	7 2 2 2		.629	.995	24.51	2026.5		184.2
B22a	Second Section	0 21.880	1	1	.569	.935	5 22.74	1866.7	9.06	170.6
B22	x 73.	0 22.250	8.545	.415	.575	-		1796.7		161.5
		5 22.120	8.520				8 19.84		9.08	2 00 00 00
Frank of the second second		5 22.000			-		18.38		9.02	
		0 21.880					3 17.14		8.92	
B22	x 54.	5 21.750	8.490	.360	.325	.66	3 16.04	1232.6	8.77	110.0

### BETHLEHEM SHAPES



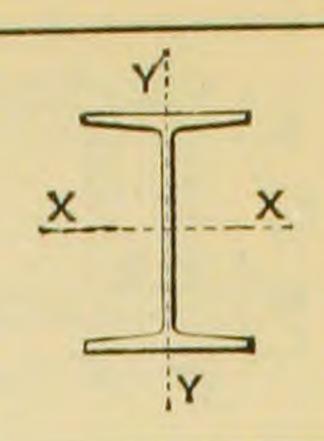
	AX	IS Y-Y		STREN	ENTS OF STH IN OUNDS	MOMEN RESISTA FOOT P	NCE IN		Weight
Ine	ment of ertia, ches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	Foot, Pounds
1	I'	r'	S'	C	C'	R	R'		
38	32.5	2.96 2.94	57.6 53.7	4,456,000	3,961,000 3,735,000	557,000 525,200		151,900 136,600	140.0
100	55.6	2.92	50.0	3.959.000	3,519,000		439,900	125,100	132.0
2	77.5	2.71	45.2	3,848,000	3,420,000	481,000	427,500 400,900	125,500 110,500	
	56.3	2.69	41.9	2 280 000	3,207,000 $3,005,000$		375,600	99,400	
	36.1	2.67	38.7	3 202 000	2,847,000	000	355,800	93,700	107.0
	$\frac{20.0}{32.9}$	2.64	36.1		2,654,000		331,800	117,300	~~ ~
	24.9	2.06		2,841,00	0 2,526,000	355,200	315,700	107,900	~~~
	17.1	2.04	011	2,703,00	0 2,402,000	337,900	300,300	100,500	95.5
1	04.9	1.99	22.1	2,575,00	0 2,289,00 0 2,138,00	0 321,900 0 300,700	286,100 267,300	89,800 84,200	~ 1 2
	95.8	1.96	20.2	2,100,00	0,2,200,00				70 -
- 1	81.2	1.87	18.0	2,258,00	0 2,007,00	0 282,300	0000	1	
	74.7			2,109,00	001,875,00	0   263,600			
	67.4		15.0	1,964,00	0 1,746,00	0 245,500	218,200	61,900	10.0
_									
	339.3	2.95	5 51.8	3,755,00	00 3,337,00	00 469,300			132.0
	312.6			3,518,00	00 3,127,00	00  439,800	221200		124.0
	286.0			3.278.00	00 2.914.00	00  409,700		- 1 1	116.0
	261.9	2.8		3,059,00	00 2,719,00	382,400	339,900		101.0
	238.1	2.8	3   36.7	7 2,841,0	00 2,526,00	355,200	310,100	00,000	101.0
		0.0	1 01	7 2 560 0	00 2 276 00	00 320,100	284,500	106,200	96.5
	115.	- 1		6 2 375 0	$\begin{array}{c c} 00 & 2,276,00 \\ 00 & 2,111,00 \end{array}$				0 89.0
.	95.8			7 2 211 0	00 1,965,0		212 200	82,20	
	87.			9 2.048.0	00 1,820,0			72,20	0 77.0
	01.						215,300	69,00	0 73.0
	69.		-	2 1,938,0	00 1,723,0	$\begin{vmatrix} 00 & 242,300 \\ 00 & 222,100 \end{vmatrix}$	1 100		
	61.			0 1,777,0	$\begin{vmatrix} 00 & 1,579,0 \\ 00 & 1,450,0 \end{vmatrix}$		- 1 000	- 1 -0	
	55.	- 1		5 1,001,0	000 1,430,0	00 187,00		51,30	0 58.0
	48. 42.		32 9	95 1.360.0	000 1,209,0	00 170,00	0   151,100		0 54.5
	12.		-						
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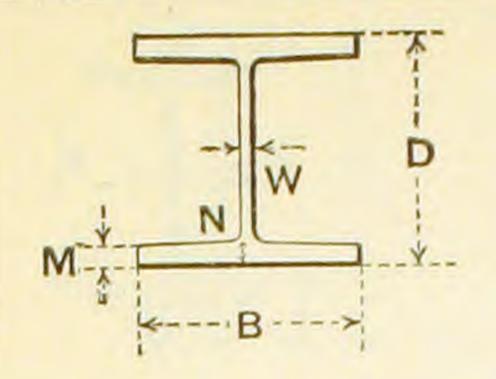
### BETHLEHEM SHAPES

	Weight	Nominal Depth	Width		INCH		Area	A	XIS X-	×
Section Number	per Foot, Pounds	of Beam, Inches	of Flange, Inches	Web	Fla	nge	Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modulus Inches <sup>3</sup>
		D	В	W	М	N		1	r	S
G20a z G20a z	x 135.0	the same of the sa	$12.780 \\ 12.750 \\ 12.720 \\ 12.690$	.690 .660 .630 .600			41.71	3134.9 2960.6 2788.9 2607.3	8.46 8.43 8.39 8.36	311.6 296.1 280.6 264.0
G20 :	x 113.0 x 107.0	20.120 $20.000$ $19.880$ $19.750$	$12.000 \\ 11.980$	.560	.710 .650	1.187 $1.127$	33.20 31.36	2528.0 2362.8 2206.5 2034.4	8.47 8.44 8.39 8.35	251.3 $236.3$ $222.0$ $206.0$
B20a	x 73.0	20.090 20.000 19.880	8.905 8.875 8.855	.430 .410	.580 .520	.932 .872	21.58 20.12	1585.5 1485.0 1366.0	8.31 8.30 8.24	157.8 148.5 137.4
B20 B20	x 62.0 x 59.5	20.120 20.060 20.000 19.880	8.025 8.015 8.000 8.000	.390 .375	.515	.834	18.25 17.47	1295.1 1239.8 1181.5 1086.1	8.27 8.24 8.22 8.11	128.7 123.6 118.2 109.3
G18 G18	x 92.0 x 86.0	18.120 18.000	11.770 $11.750$	.460	.584	1.055 $.995$	27.13 25.35	1767.7 1628.5 1503.6 1380.7	7.75	193.7 179.8 167.1 154.4
B18a B18a	x 69.0 x 64.5	18.120 18.000 17.880 17.750	8.750 8.730	.420	.585	.932	20.37	1249.2 1153.7 1059.7 960.3	7.53	137.9 128.2 118.5 108.2
B18 B18	x 52.0 x 49.0	18.120 18.060 18.000 17.940	7.525 $7.500$	.355	.465 .435	.764 .734	16.06 15.34 14.44 13.90	896.1 851.7 802.8 764.1	7.47 7.45 7.46 7.42	98.9 94.3 89.2 85.2

### BETHLEHEM SHAPES



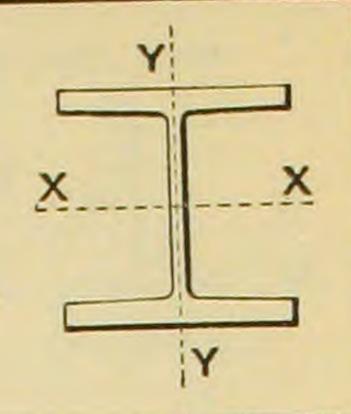
A)	KIS Y-Y		COEFFICIE STRENG FOOT PO	TH IN	RESISTA	NTS OF NCE IN OUNDS	Maxi- mum Safe	Weight
Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	Foot, Pounds
I'	r'	S'	C	C'	R	R'		
384.6 361.0 337.7 313.0	2.96 2.94 2.92 2.90	56.6	3,553,000 3,367,000	3,324,000 3,158,000 2,993,000 2,816,000	444,100 420,900	394,700 374,100 352,000	155,400 145,400 135,500 125,700	$142.0 \\ 135.0 \\ 127.0$
260.2 $240.8$ $222.4$	2.72 $2.69$ $2.66$	43.3 $40.1$ $37.1$	2,835,000	2,680,000 $2,520,000$ $2,368,000$	354,400	335,100 315,000 296,000	123,500 113,700 106,900	113.0 107.0
202.1	2.63	33.8	2,472,000	2,197,000	309,000	274,700	97,300 82,200 72,900	78.0
78.5 71.0	1.91		1,649,000	1,584,000	206,100	198,000 183,200 171,600	66,600	68.5
54.3 51.5 48.6 43.5	1.67	13.5 12.9 12.2 10.9	1,483,000	0 = 1,373,000 $0 = 1,318,000$ $0 = 1,260,000$ $0 = 1,166,000$	$0   185,400 \\ 0   177,200$	164,800 157,500	60,700 56,200	62.0 59.5
211.3 192.2 174.9	2.66 $2.63$	32.7 29.8	2,157,00 2,005,00	0 2,066,00 0 1,917,00 0 1,782,00	$\begin{array}{c c} 0 & 269,600 \\ 0 & 250,600 \end{array}$	239,700 222,800	80,200	92.0
157.8 82.9 75.6 68.4	1.95	18.9	1,655,00 1,538,00 1,422,00	0   1,647,00 $0   1,471,00$ $0   1,367,00$ $0   1,264,00$	206,800 192,300 177,800	183,800 170,900 158,000	74,300 68,400 62,600	74.0 69.0 64.5
41. 38. 36.	$ \begin{array}{c cccc} 1 & 1.60 \\ 7 & 1.59 \\ 1 & 1.59 \end{array} $	10.9 10.3 10.3 9.6	1,187,00		148,400 100 141,500 100 133,800	131,900 125,800 118,900	54,30 50,20 43,70	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
34.	1.5	3.0	1,022,0					



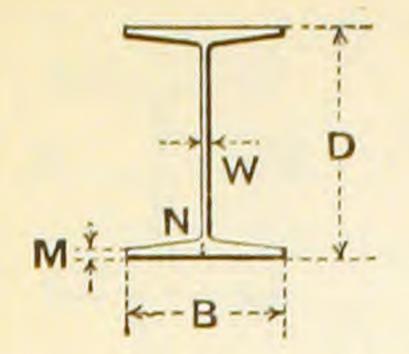
### BETHLEHEM SHAPES

		Nominal	Width		ICKNES		Area	A	(IS X-)	
Section	Weight per Foot, Pounds	Depth of Beam, Inches	of Flange, Inches	Web	Flan	nge	of Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modulus, Inches <sup>3</sup>
		D	В	W	M	N		I	r	S
H16	v 300 0	18.250 18.063	16.640	$\frac{1.880}{1.820}$	2.857 $2.763$	3.005 $2.911$	$121.48 \\ 117.26$	6121.5 5834.0	7.14 7.10 7.05 7.01	696.0 670.8 646.0 621.4
HIG	× 370 0	17.875 17.688 17.500	16.520	1.700	2.576	2.724	108.90	5280.2	6.96 6.92	597.1 573.0
H16b	x 363.0	17.313	16.760	1.940	2.388	2.536	106.86	4909.6	6.78	567.2
H16	x 328.0 x 314.0	17.313 17.125 16.938 16.750	16.340 $16.280$	$1.520 \\ 1.460$	2.294 $2.201$	2.443 $2.349$	96.53	4254.5	6.87 6.83 6.78 6.74	549.2 525.7 502.4 479.8
H16	x 288.0	16.750	16.230	1.360	2.013	2.161	84.69	3788.4	6.69	457.5
H16a	x 293.0	16.375	16.460	1.640	1.919	2.068	86.24	3685.1	6.54	450.1
H16 H16	x 265.0 x 256.0	16.375 16.250 16.125 16.000	16.080 16.040	$\begin{vmatrix} 1.260 \\ 1.220 \end{vmatrix}$	1.857 $1.794$	$\frac{2.005}{1.943}$	78.00 75.35	3560.7 $3412.4$ $3266.7$ $3123.7$	6.64 6.61 6.58 6.55	434.9 420.0 405.2 390.5
H16 H16 H16 H16	x 238.0 x 230.0 x 221.0 x 212.0	15.875 15.750 15.625 15.500	$15.960 \\ 15.930 \\ 15.900 \\ 15.860$	1.140 1.110 1.080 1.040	1.669 $1.607$ $1.544$ $1.482$	1.818 $1.755$ $1.693$ $1.630$	70.07 67.60 65.14 62.53	2983.4 2848.9 2716.9 2584.1	6.53 6.49 6.46 6.43	375.9 361.8 347.8 333.4
H16 H16	x 195.0 x 186.0	15 250	$\begin{vmatrix} 15.780 \\ 15.740 \end{vmatrix}$	.960 .920 .880	1.357 $1.294$ $1.232$	1.505 1.443 1.380	57.35 54.77 52.20	2453.9 2326.1 2200.9 2078.0	6.34 6.31	277.1
H16 H16	x 160.0 x 151.0	0 14.875 $0 14.750$ $0 14.625$ $0 14.500$	15.620 15.580	.800	1.107	1.255	47.10 44.56	1957.6 1839.5 1723.8 1610.4	6.25	263.2 249.4 235.7 222.1

### BETHLEHEM SHAPES



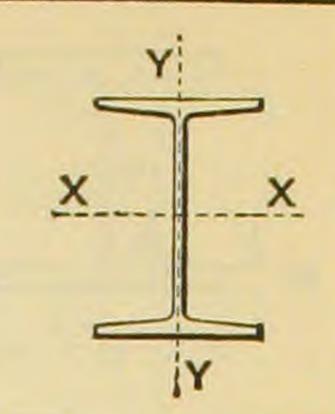
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	AXI	S Y-Y		COEFFICIE STRENG FOOT PO	TH IN	MOMEN' RESISTA FOOT PO	NCE IN	Maxi- mum Safe	Weight
Y	Moment of Inertia,	of Gyra- tion,	Modu-	Stress of 18,000 Lbs.	Stress of 16,000 Lbs.	Stress of 18,000 Lbs. per Sq. In.	Stress of 16,000 Lbs. per Sq. In.	Web,	Foot, Pounds
2355.9         4.33         281.1         8,352,000 (7,424,000) (7,424,000) (1,006,000)	I'	r'	S'	C	C'	R	- It		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	2257.2 2160.3	4 29	270.3 259.6	8,050,000 $7,752,000$	6,890,000	969,000	894,500 861,300 828,500	411,700 394,500 377,500	413.0 399.0 384.0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1971.7	4 26	238.7	7,165,000	6,369,000	895,600			370.0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1914.5	4.23	228.5	6,806,000	6,050,000	850,800			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1701.8	4.20	208.3	6,308,000	0 5,607,000	753,600	700,900	312,400	328.0
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	1533.2	4.16	188.9	5,758,00	0 5,118,00	0 719,700	639,700	283,400	0 301.0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$							600,100		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1316.8	3 4.11	163.8	8 5,040,00	00 4,480,00	630,00	0 560,000	245,70	0 265.0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			157.	4,802,00 $4,686,00$	00 4,322,00			226,60	0 247.0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1159.8 1111.0 1062.	8 4.07 0 4.05 7 4.04	145. 139. 133.	3 4,510,00 5 4,341,00 7 4,173,0	$ \begin{array}{c cccc} 00 & 4,009,00 \\ 00 & 3,859,00 \\ 00 & 3,709,0 \end{array} $	$     \begin{array}{c c}       00 & 563,80 \\       00 & 542,70 \\       00 & 521,60     \end{array} $	00 482,400 00 463,700	$\begin{vmatrix} 209,80 \\ 202,50 \end{vmatrix}$	$\begin{vmatrix} 00 & 230.0 \\ 00 & 221.0 \end{vmatrix}$
867.7 3.98 110.3 3,492,000 3,104,000 415,600 369,400 158,400 177 820.7 3.96 104.5 3,325,000 2,955,000 394,800 350,900 149,900 169	963. 915.	9 4.01 4.00	$ \begin{array}{c c} 1 & 121 \\ 1 & 116 \end{array} $	$\begin{array}{c c} 9 & 3,830,0 \\ 0 & 3,661,0 \end{array}$	$\begin{array}{c c} 00 & 3,405,0 \\ 00 & 3,254,0 \end{array}$	00 478,80 00 457,60	$\begin{array}{c c} 00 & 425,60 \\ 406,80 \\ \end{array}$	$ \begin{array}{c c} 0 & 184,50 \\ 0 & 175,70 \end{array} $	$\begin{array}{c c} 00 & 203.0 \\ 00 & 195.0 \\ \end{array}$
227 6 14 6 10 10 10	820.	.7 3.9	6 104 5 98	.5 3,325,0 .9 3,158,0	$\begin{vmatrix} 000 & 2,955,0 \\ 000 & 2,808,0 \end{vmatrix}$	000 415,6 000 394,8	00   369,40 $00   350,90$	0 158,40	00   177.0 $00   169.0$
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	728 683	.5 3.9 .4 3.9	$\begin{array}{c c} 3 & 93 \\ 2 & 87 \end{array}$	3.3 2,993,0 7.7 2,829,0	$\begin{array}{c c} 000 & 2,661,0 \\ 000 & 2,514,0 \end{array}$	374,1 $374,1$ $353,6$	314,30	0 133,4	00 151.0



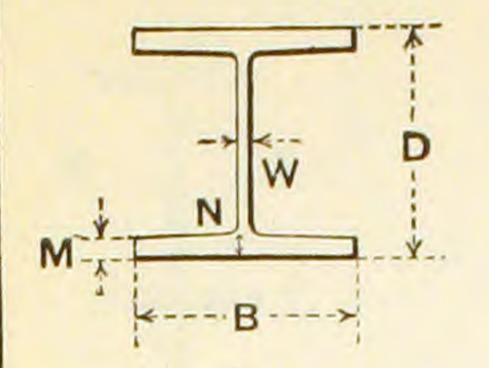
### BETHLEHEM SHAPES

	Weight	Nominal Depth	Width		IICKNE		Area	A	XIS X-	×
Section Number	per Foot, Pounds	of Beam, Inches	of Flange, Inches	Web	Fla	nge	of Section, Square Inches	Moment of Inertia,	Radius of Gyra- tion,	Section Modulus, Inches <sup>3</sup>
		D	В	w	M	N		Inches <sup>4</sup>	Inches	S
	x 87.0 x 81.0	$16.120 \\ 16.000$	$11.565 \\ 11.530 \\ 11.500 \\ 11.470$		.589	$1.051 \\ .991$	23.82	1341.4 $1230.8$ $1131.3$ $1033.6$		165.1 $152.7$ $141.4$ $130.2$
B16a : B16a :	x 66.0 x 60.5	$16.250 \\ 16.120 \\ 16.000 \\ 15.880$	8.530 8.500	.420	.599	.936 .876	21.07 19.40 17.89 16.63	973.5 888.4 812.1 742.3	$6.77 \\ 6.74$	119.8 110.2 101.5 93.5
B16 :	x 45.0 x 40.0	$16.250 \\ 16.120 \\ 16.000 \\ 15.810$	7.285 $7.250$	.330	.418	.708 .648	14.78 13.26 11.83 10.29	669.0 594.5 526.2 435.8	6.69 6.67	82.3 73.8 65.8 55.1
G15b :	x 141.0 x 135.0	$15.000 \\ 14.880$	$11.750 \\ 11.720$	.800 .770	$\frac{1.090}{1.030}$	$\frac{1.546}{1.486}$	41.44 39.58	1685.4 1596.8 1509.9 1415.6	6.21 6.18	222.9 212.9 202.9 191.9
G15a : G15a :	x 105.0 x 99.0	$15.000 \\ 14.880$	$11.290 \\ 11.250 \\ 11.220 \\ 11.190$	.600 .570	.785 .725	1.229 $1.169$	30.80 29.00	1319.3 $1231.3$ $1147.7$ $1090.2$	6.32 6.29	174.5 $164.2$ $154.3$ $147.3$
G15	x 74.0	15.000	$10.790 \\ 10.750 \\ 10.730$	.440	.510	.940	23.66 $21.76$ $20.18$	977.4 892.7 815.3		129.3 119.0 109.6
			7.500					799.5		106.6
B15a : B15a : B15a :	x 59.5 x 54.5 x 50.5	15.120 15.000 14.880 14.750	7.040	.450 .410 .385	.660 .600 .540	.935 .875 .815	19.09 17.49 16.05 14.84 13.63	676.2 617.0 563.3	6.16	104.1 89.4 82.3 75.7 68.9
B15 B15	x 40.0 x 38.5	15.090 15.030 15.000 14.910			.445 .415 .400 .355	.684	12.50 11.80 11.37 10.61	492.0 463.3 447.6 410.9	6.27 6.27 6.27 6.22	65.2 61.6 59.7 55.1

#### BETHLEHEM SHAPES



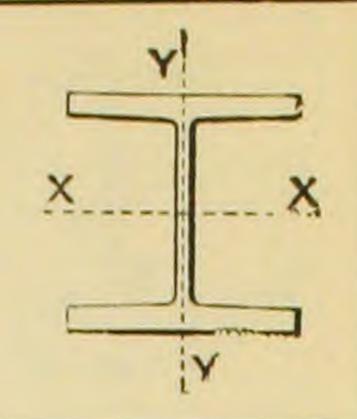
	14							
A	XIS Y-Y		COEFFICI STRENC FOOT P	STH IN	RESISTA	NTS OF ANCE IN POUNDS	Maxi- mum	\A/-:
Moment of Inertia, Inches <sup>4</sup>	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Safe Shear on Web, in Pounds	Weight per Foot, Pounds
199.9 181.3 164.6 148.1	2.68 $2.66$ $2.63$ $2.60$		1,832,000 1,697,000	1,761,000 1,629,000 1,508,000 1,389,000	229,000 212,100	$220,100 \\ 203,600 \\ 188,500 \\ 173,600$	83,200 74,000 66,200 58,400	87.0 81.0
79.0 71.2 64.3 57.8	1.94 1.92 1.90 1.86	16.7 15.1	1,323,000 1,218,000	1,278,000 1,176,000 1,083,000 997,200	165,300 152,300	159,800 147,000 135,400 124,700	75,200 66,100 58,300 54,300	66.0 60.5
36.6 31.9 27.6 21.4	1.57 1.55 1.53 1.44	$10.01 \\ 8.75 \\ 7.61 \\ 5.92$	885,100 789,200	786,700 701,500	110,600 98,660	109,800 98,340 87,690 73,500	52,000 43,200 34,900 32,200	45.0 40.0
347.5 328.5 309.7 289.1	2.83 $2.82$ $2.80$ $2.78$	59.0 55.9 52.9 49.5	2,555,000 2,435,000	2,378,000 $2,271,000$ $2,165,000$ $2,047,000$	319,400 304,400	297,200 283,900 270,600 255,900	150,600 144,000 137,500 129,200	$141.0 \\ 135.0$
231.3 214.4 198.5 187.4	2.66 $2.62$ $2.60$	38.1 35.4	1,970,000 1,851,000	1,862,000 $1,751,000$ $1,645,000$ $1,571,000$	246,300 231,400	232,700 218,900 205,700 196,400	$115,800 \\ 106,100 \\ 98,600 \\ 91,400$	105.0 99.0
143.1 128.9 115.8 60.9	2.46 2.43 2.40 1.70	21.6	1,428,000 1,315,000	1,379,000 1,270,000 1,169,000 1,137,000	178,500 164,400	172,400 158,700 146,100 142,100	78,400 68,700 63,700 87,500	74.0 69.0
108.6	2.39	20.3		1,111,000		138,800	56,700	
42.8 38.6 34.7 30.8	1.56 1.55 1.53 1.50	12.2 11.0 9.96 8.85		877,600 807,600	123,400 113,600	119,300 109,700 101,000 91,880	71,300 61,600 55,400 50,500	54.5 50.5
26.9 $25.1$ $24.1$ $21.7$	1.47 1.46 1.46 1.43	7.93 7.42 7.15 6.45	739,700 716,100	657,500	92,470 89,520	86,940 82,190 79,570 73,480	41,700 37,100 33,800 31,400	38.5



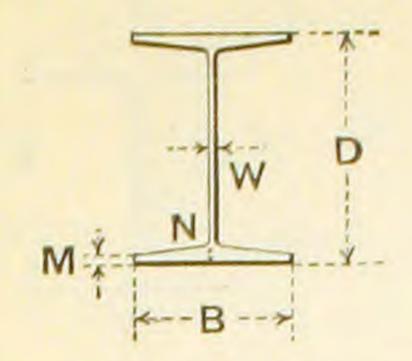
### BETHLEHEM SHAPES

		Nominal	Width		ICKNE		Area	A	XIS X-	×
Section Number		Depth of Beam, Inches	of Flange, Inches	Web	Flai	nge	of Section, Square Inches	Moment of Inertia, Inches <sup>4</sup>	Radius of Gyra- tion, Inches	Section Modulus, Inches <sup>3</sup>
		D	В	W	M	N			r	S
H14	v 208 0	16.875	15.610	1.390	2.175	2.317	87.63	4011.3	6.77	475.4
H14	x 289 0	16.750						3857.7	6.74	460.6
H14	x 280.0	16.625	15.530	1.310	2.050	2.192	82.39	3706.9	6.71	445.9
H14	x 271.0	16.500	15.490	1.270	1.987	2.130	79.79	3558.8	6.68	431.4
H14	x 262.0	16.375	15.450	1.230	1.925	2.067	77.20	3413.4	6.65	416.9
								3270.6	6.62	402.5
1114	x 204.0	16 125	15.370	1.150	1.800	1.942	72.05	3130.4	6.59	388.3
H14	x 240.0	16.000	15.330	1.110	1.737	1.880	69.49	2992.9	6.56	374.1
H14	× 227 0	15.875	15.290	1.070	1.675	1.817	66.94	2857.8	6.53	360.0
H14	x 219.0	15.750	15.250	1.030	1.612	1.755	64.40	2725.3	6.51	346.1
								2595.4	6.48	332.2
		15.500			1.487	1.630	59.50	2470.9	6.44	318.8
		15.375						2345.8		
		15.250						2223.0		291.5
		15.125						2102.6		278.0
		15.000			1.237	1.380	49.51	1984.6	6.33	264.6
		14.875						1874.4		252.0
		14.875	1					1793.8		241.2
H14	v 147 0	14 750	14.230	.740	1.120	1.255	43.25	1685.3	6.24	228.5
H14	x 139 0	14.625	14.190	.700	1.058	1.192	40.88	1578.9	6.21	215.9
H14	x 131.5	14.500	14.160	.670	.995	1.130	38.68	1477.3	6.18	203.8
H141	x 149.0	14.125	14.900	1.410	.808	.942	43.82	1379.1	5.61	195.3
H14	x 123.5	14.375	14.120	.630	.933	1.067	36.33	1375.1	6.15	191.3
		14.250		.590	.870	1.005	34.00	1275.1	6.12	179.0
		14.125		.550				1177.2		
H14	x 100.0	14.000	14.000	.510				1081.2		
H14	x 92.0	13.875	13.960	.470	.683	.817	27.05	987.4	6.04	142.3
								T		

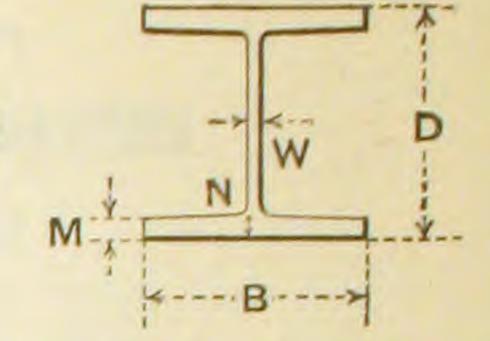
### BETHLEHEM SHAPES



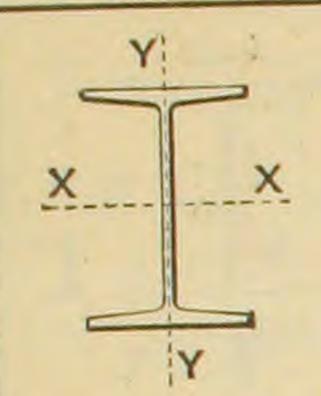
A	CIS Y-Y		COEFFICIE STRENG FOOT P	TH IN	MOMEN RESISTA FOOT P	NCE IN	Maxi- mum Safe	Weight
Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	per Foot, Pounds
I'	r'	S'	C	C'	R	R'		
1406.5	4.01	180.2	5 705 000	5,071,000	713,100	633,900	281,500	298.0
1356.1				4,913,000			271,400	
1306.4	3.98			4,757,000		594,600		-
1257.3	3.97			4,601,000			251,500	
1209.0	3.96			4,447,000		555,900		
				4,294,000		536,700	232,100	254 0
1161.2	3.94			4,234,000		517,700	-	
1114.2	3.93			3,990,000	The second section of	498,800		
1022.0	3.91			3,840,000	,	480,100		
976.9	3.89			3,691,000		461,400		
932.4	3.88	1	1	3,544,000		442,900		
890.3	3.87			3,401,000			178,600	
846.9	3.86			3,255,000		406,900	169,700	
804.2	3.84			3,110,000		388,700		
762.1	3.83			2,966,000		370,700		
720.6	3.82			2,823,000		352,800	144,000	
682.5	3.80			2,688,000		336,000		
590.6	3.60	82.8		2,573,000		321,600	139,200	
555.5				2,438,000		304,700	131,000	
520.9		1		2,303,000		287,900		
488.0	3.55	68.9	2,445,000	2,173,000	305,600	271,700	116,600	131.5
468.8	3.27	62.9	2,343,000	2,083,000	292,900	260,400	239,000	149.0
454.4	3.54	64.4	2,296,000	2,041,000	287,000	255,100	108,600	123.5
421.4				0 1,909,000		238,600	99,260	115.5
388.9	3.50			0 1,778,000		222,200	90,020	107.5
356.9	3.49	51.0	1,854,00	0 1,648,000	0 231,700	206,000	80,860	100.0
325.5	3.47	46.6	1,708,00	0 1,518,00	0 213,500	189,800	71,800	92.0



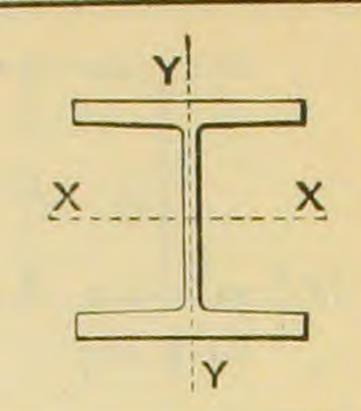
### BETHLEHEM SHAPES



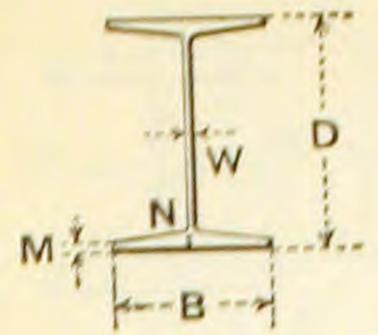
	147-1-1-1	Nominal	Width		ICKNE		Area	A	XIS X-	<
Section Number	Weight per Foot, Pounds	Depth of Beam, Inches	of Flange, Inches	Web	Flai	nge	of Section, Square Inches	Moment of Inertia,	Radius of Gyra- tion,	Section Modulus Inches <sup>8</sup>
		D	В	W	М	N		Inches <sup>4</sup>	Inches	S
$H^{\frac{1}{4}}$	x 90.0	14.000	12.120	.510	.764	.880	26.52	956.7	6.01	136.7
H14	x 84.0	13.750	13.920	.430	.620	.755	24.76	895.5	6.01	130.2
$H^{\frac{14}{12}}$	x 83.0	13.875	12.080	.470	.701	.817	24.45	874.2	5.98	126.0
$H^{\frac{1}{4}}$	x 76.0	13.750	12.040	.430	.639	.755	22.39	793.5	5.95	115.4
$H_{10}^{14}$	x 73.5	13.875	10.120	.470	.721	.817	21.66	753.3	5.90	108.6
$H^{\frac{1}{4}}$	x 69.0	13.625	12.000	.390	.576	.692	20.34	714.6	5.93	104.9
$H_{\frac{1}{1}0}^{\frac{1}{4}}$	x 67.5	13.750	10.080	.430	.658	.755	19.85	684.3	5.87	99.5
								616.9		
$H^{\frac{1}{8}}$	x 58.5	13.750	8.120	.430	.678	.755	17.23	572.2	5.76	83.2
$H_{\frac{1}{1}0}^{\frac{1}{4}}$	x 55.0	13.500	10.000	.350	.533	.630	16.25	551.0	5.82	81.6
$H^{-\frac{1}{8}4}$	x 53.5	13.625	8.080	.390	.616	.692	15.67	516.2	5.74	75.8
$H^{-\frac{1}{8}}$	x 48.0	13.500	8.040	.350	.553	.630	14.12	461.5	5.72	68.4
B14	x 42.0	14.250	6.825	.340	.443	.713	12.46	436.5	5.92	61.3
H-1-4	x 43.0	13.375	8.000	.310	.491	.567	12.58	408.2	5.70	61.0
B14	x 37.5	14.120	6.790	.305	.378	.648	11.07	383.7	5.89	54.3
B14	x 33.0	14.000							5.87	47.8
B14	x 30.0	13.880	6.750	.265	.258	.528	8.89	294.9	5.76	42.5



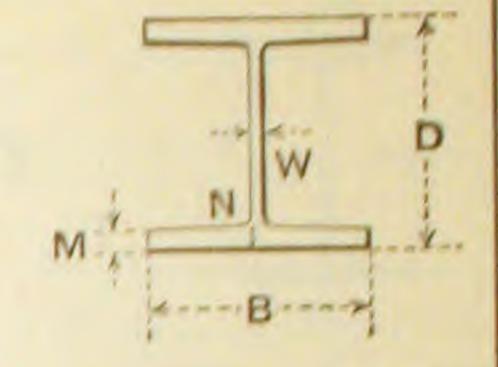
### BETHLEHEM SHAPES



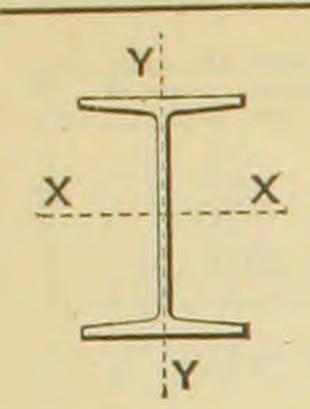
	AX	IS Y-Y		COEFFICIE STRENG FOOT PO	TH IN	MOMEN RESISTA FOOT P	NCE IN	Maxi- mum Safe	Weight
In	ment of ertia, ches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	Foot, Pounds
	I'	r'	S'	· C		R	<u>R'</u>		
2	35.8	2.98	38.9	1,640,000	1,458,000	205,000	182,200	80,860	90.0
2	94.5	3.45	42.3	1,563,000	1,389,000	195,400	173,700	62,840	84.0
2	15.1	2.97	35.6	1,512,000	1,344,000	189,000	168,000	71,800	83.0
1	94.7	2.95	32.3	1,385,000	1,231,000	173,100	153,900	62,840	76.0
1	29.1	2.44	25.5	1,303,000	1,158,000	162,900	144,800	71,800	73.5
1	174.7	2.93	29.1	1,259,000	1,119,000	0 157,300	139,900	54,000	69.0
	116.8	2.43	23.2	1,194,000	1,062,00	0 149,300	132,700	62,850	67.5
	104.8	2.41	20.9	1,087,000	965,90	0 135,800	120,700	54,000	61.5
	62.4	1.90	15.4	998,700	887,70	0 124,800	111,000	62,850	58.5
	93.1	2.39	18.6	979,50	870,70	0 122,400	108,800	45,310	55.0
	56.0	1.89	13.9	909,20	0 808,20	00 113,700	101,000	54,000	53 5
	49.7	1.88	3 12.4	820,50	0 729,30	102,600	91,170	45,310	48.0
	27.3	3 1.48	8 8.0	735,20	653,50	91,900	81,690	44,400	42.0
	43.0	3 1.8	6 10.9	732,50	651,10	91,560	81,380	36,830	43.0
	23.	4 1.4	6 6.9	91 652,20	579,70	00 81,520	72,460	36,600	
	19.	9 1.4	3 5.	90 573,00	509,4	00 71,63	63,670		
	16.	9 1.3	8 4.	99 510,0	00 453,3	00 63,74	56,660	27,90	0 30.0



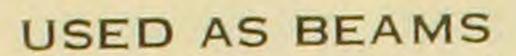
### BETHLEHEM SHAPES

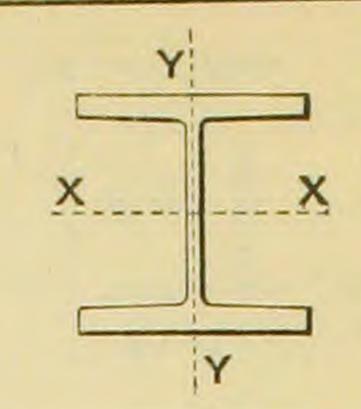


		Nominal	Width		INCHE		Area	A	XIS X-	<
Section Number	Weight per Foot, Pounds	Depth of Beam, Inches	of Flange, Inches	Web	Flar	ge	of Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modulus Inches <sup>3</sup>
		D	В	w	М	N		1	r	S
		11000	10.000	1 000	1 000	1 909	55.91	1780.9	5.64	254.4
H12	x 190.0	14.000	12.620		1.692			7		244.4
H12	x 183.0	13.875	12.580	1.000	1.567	1 682	51 70	1613.9	5.58	234.7
H12	x 176.0	13.750	12.550	0.020	1.505	1.690	10.68	1532.0	5.55	224.9
		13.625			1 449	1.558	47.57	1451.9	5.52	215.1
H12	x 162.0	13.500	12.470							
H12	x 154.5	13.375	12.430					1373.5	The same of the same of	205.4
H12	x 147.5	13.250	12.390	1				1296.9	The second second	195.8
H12	$\times 140.5$	13.125	12.350				41.32	1222.1	5.44	186.2
H12	x 133.5	13.000	12.310				39.26		5.41	176.8
H12	x 126.5	12.875	12.270	.740	1.130	1.245	37.21	1077.4	5.38	167.4
H12	v 119 5	12.750	12.230	.700	1.067	1.183	35.16	1007.5	5.35	158.0
		12.625					33.25	941.0	5.32	149.1
		12.500					31.23	874.3	5.29	139.9
		12.375					29.21	809.2	5.26	130.8
		12.250	IV		.817	.933	27.21	745.7	5.23	121.7
					755	870	95.91	683.6	5.21	112.8
H12	x 85.5	12.125	12.040	170	609	808	23.23	623.1		103.9
H12	x 79.0	12.000	12.000	.970	.002	*000	20.20	020		
G12a	x 76.	5 12.120	10.290	.510	.620	1.027	22.50	594.2	5.14	98.1
H12	x 72.	5 11.875	11.960	.430	.630	.745	21.25	564.1	5.15	95.0
		5 12.000	1							
		0 12.000								1
1	_	5 11.750					Y			
		0 11.880								

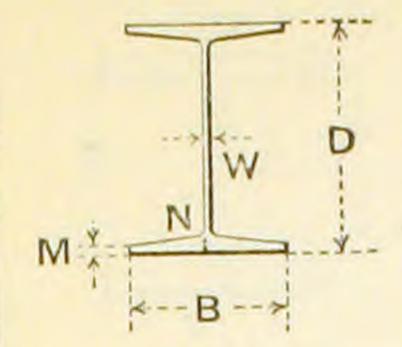


### BETHLEHEM SHAPES

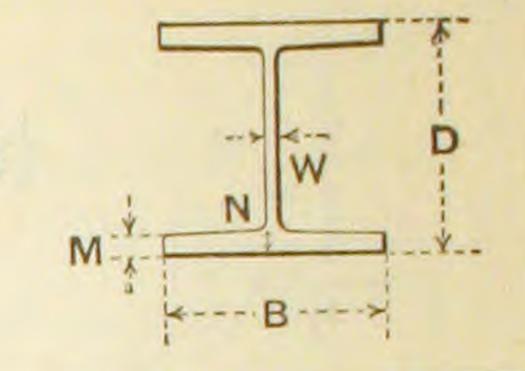




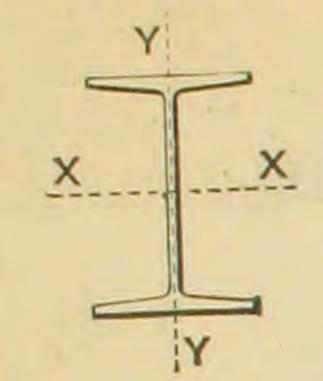
Weight	Maxi- mum Safe	NCE IN	MOMEN RESISTAL FOOT PO	TH IN	STRENG FOOT PO		IS Y-Y	AX
Foot, Pounds	Shear on Web, in Pounds	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 8,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 8,000 Lbs. per Sq. In.		Radius of Gyra- tion, Inches	Moment of Inertia, Inches
		R'	R	C'	C	S'	$\mathbf{r}'$	I'
190.0	183,100	339,200	381,600	2714 000	0 0 0 0 0 0 0	01 5	0.00	
		325,800		2,714,000			3.22	578.7
	168,300			2,607,000			3.20	552.4
	160,200			2,504,000			3.19	527.7
	152,300			2,399,000			3.18	502.2
			522,000	2,294,000	2,581,000	76.5	3.17	477.0
	144,500		308,100	2,191,000	2,465,000	72.8	3.15	452.3
	136,700		293,600	2,088,000	2,349,000	69.1	3.14	428.1
	129,200		279,300	1,986,000	2,235,000	65.5	3.13	404.2
	121,700	235,700	265,100	1,885,000	2,121,000	61.9	3.11	380.8
126.5	114,300	223,100	251,000	1,785,000	2,008,000	58.3	3.10	357.7
	107,100		237,100	1,686,000	1,897,000	54.8	3.09	335.1
	101,500	198,800		1,590,000			3.07	313.7
106.0	94,500	186,500		1,492,000		48.0	3.06	291.8
99.5	87,620	174,400		1,395,000		44.6	3.04	270.3
92.5	80,850	162,300		1,299,000		41.3	3.03	249.2
85.5	73,340	150,400		1,203,000		38.0	3.01	228.5
79.0	65,480	138,500		1,108,000		34.7	2.99	208.2
76.5	73,900	130,700		1,046,000			2.42	132.1
72.5	57,710	126,700	142,500	1,013,000	1,140,000	31.5	2.98	188.2
70.5	66,000	120,800	135,900	966,300	1,087,000	23.4	2.40	119.7
70.0	65,480	119,700	134,700	957,900	1,078,00	25.2	2.49	127.3
65.5	50,040	115,000	129,300	919,700	1,035,00	28.3	2.96	168.6
66.0	61,900	111,500	125,500	892,40	1,004,00	21.2	2.37	108.3



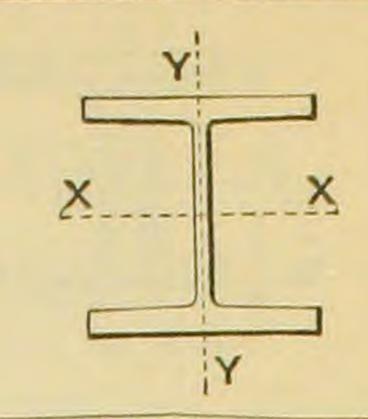
## PROPERTIES OF BETHLEHEM SHAPES



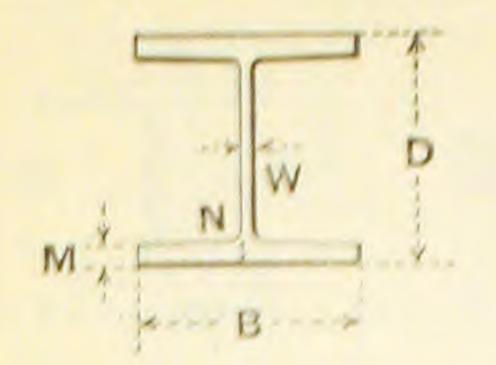
		Nimainal	Width		CKNES		Area	A	XIS X-X	
Section		Nominal Depth of Beam, Inches	of Flange, Inches	Web	Flan	ge	of Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion,	Section Modulus, Inches <sup>3</sup>
		D	В	W	M	N		I	Inches	S
W 10		11.875		.430	.649	.745	18.85	488.2	5.09	82.2
G12	x 61.0	12.120	10.030	.410	.465	.866	17.92	483.6	5.20	79.8
$H_{\frac{1}{1}0}^{\frac{1}{2}}$	x 58.0	11.750	10.040	.390	.586	.683	17.12	438.8	5.06	74.7
G12	x 55.5	12.000	10.000	.380	.405	.806	16.35	435.6	5.16	72.6
$H^{-\frac{1}{8}}$	x 55.0	11.875	8.120	.430	.668	.745	16.27	406.9	5.00	68.5
		11.910								
		11.625								
		11.750			F .					
_		12.250								
		11.625					II.	N.		
B12	a x 44.0	12.120 $12.000$	6.780	.360	.528	.795		335.1	5.08 5.05	55.3 50.2
H 1 8	$^{2} \times 40.5$	11.500	8.000	.310	.481	.558	11.85	287.7	4.93	50.0
B12 B12 B12	x 36.0 x 31.8 x 28.0	12.250 12.120 12.000 11.880	6.555 6.525 6.500	.300	.415 .350 .290	.675 .610 .550	10.58 9.36 8.28	281.8 $245.7$ $213.6$	5.16 5.12 5.08 4.99	46.0 40.5 35.6 31.2
BJ1	2 x 18.	5 12.000	4.125	.240	.240	.402	5.44	121.5	4.73	20.2



### BETHLEHEM SHAPES



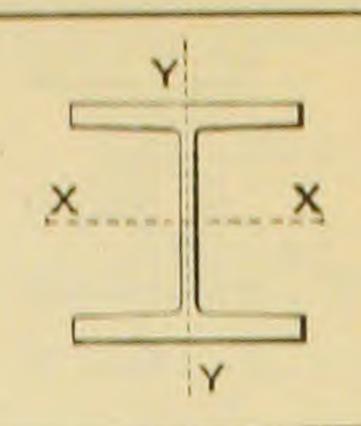
A	XIS Y-Y		STRENG FOOT P	TH IN	RESISTA FOOT P	NCE IN	Maxi- mum Safe	Weight
Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	per Foot, Pounds
I'	r'	S'	C		R			
115.1	2.47	22.8	986,600	877,000	123,300	109,600	57,710	64.0
95.9	2.31	19.1	957,700	851,300	119,700	106,400	55,300	61.0
103.2	2.45	20.6	896,300	796,700	112,000	99,590	50,040	58.0
84.9	2.28	17.0	871,100	774,300	108,900	96,790	49,300	55.5
61.5	1.94	15.2	822,400	731,000	102,800	91,380	57,710	55.0
76.9	2.25	15.4	807,200	717,500	100,900	89,690	45,300	51.5
91.5	2.44	18.3	806,700	717,000	100,800	89,630	42,470	52.5
55.1	1.93	13.6	747,800	664,700	93,470	83,080	50,040	50.5
35.1	1.57	10.29	731,200	650,000	91,400	81,250	52,800	48.5
48.9	1.92	12.2	673,800	598,900	84,220	74,860	42,470	45.5
31.1 27.6	1.55 1.53		000 400	589,900 535,500	82,960 75,310	73,740 66,940	45,800 39,900	10.00
42.8	1.90	10.7	600,400	533,700	75,050	66,710	35,030	- ALC: -
22.7 19.4 16.4	1.44	5.93	3 486,500	432,400		61,340 54,050 47,470	34,500 $28,600$ $23,900$	31.5
13.6				200 100	46,740	41,550		
3.38	3 .78	1.6	1 243,000	216,000	30,370	26,990	22,530	18.5



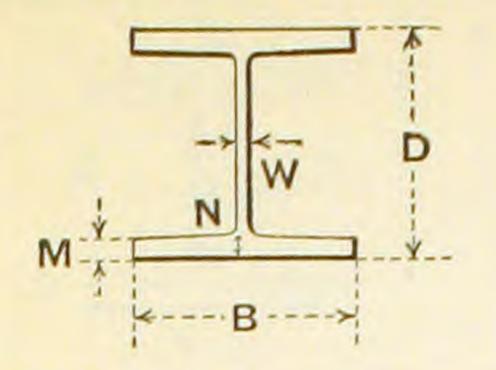
### BETHLEHEM SHAPES

	Marinist	Nominal	Width		INCHI		Area	A	XIS X->	
Section Number	Weight per Foot, Pounds	Depth of Beam, Inches	of Flange, Inches	Web	Flan	nge	of Section, Square Inches	Moment of Inertia, Inchest	Radius of Gyra- tion,	Section Modulus Inches <sup>3</sup>
		D	В	W	M	N		I	Inches	s
H 1 0	x 246.0	13.000	14.570	1.220	2.040	2.173	72.30	1916.1	5.15	294.8
			14.530	1.180	1.980	2.113	70.04	1835.8	5.12	285.1
1 64								1753.1		
4 64								1670.5		
H 1 0	x 215.0	12.500	14.430	1.080	1.790	1.923	63.27	1597.2	5.02	255.6
H 1 0	x 208.0	12.375	14.400	1.050	1.730	1.863	61.17	1525.5	4.99	246.4
		4						1448.4		
1 (4								1373.2	1	
H 1 0	x 185.0	12.000	14.290	.940	1.540	1.673	54.37	1306.3	4.90	217.7
$H_{12}^{\frac{1}{10}}$	× 177.0	11.875	14.250	,900	1.480	1.613	52.18	1239.6	4.87	208.7
H 1 0	× 170.0	11.750	14.220	.870	1.415	1,548	49.98	1170.9	4.84	199.7
* /*		11.625						1103.9		
2 14		11.500			1.290	1.423	45.62	1042.0	4.78	181.2
A. 104		11.375	1	1	1.230	1.363	43.46	981.5	4.75	172.1
2. 10		11.250			1.165	1.298	41.29	919.2	4.72	163.
H 1 0	x 133,0	11.125	14.040	.690	1.100	1.233	39.02	857.4	4.69	154.2
H10	x 136.5	11.750	10.550	.940	1.452	1.548	40.08	893.3	4.72	152.
H 1 0	x 125.0	11.000	14.000	.650	1.040	1.173	36.89	801.4	4.66	145.
H10	× 130.0	11.625	10.510				38.30	843.0	4.69	145.0
H10	x 124.0	11.500	10.470	.860			36.52	794.0	4.66	138.
1110	x 110.	11.375	10.400	.020	1.209	1,001	02.10	1 2.0.0	2,00	

### BETHLEHEM SHAPES



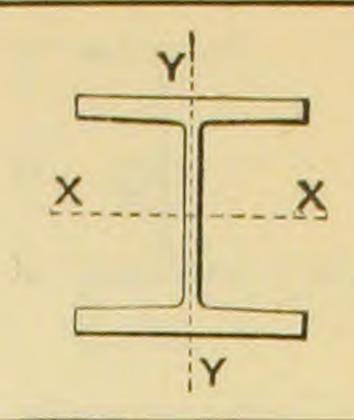
A	XIS Y-Y		STRENG FOOT P	TH IN	MOMEN RESISTA FOOT P		Maxi- mum Safe	Weight
Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	Foot, Pounds
1'	r.	S'	C	C'	R	R'		
1071.6	3.85	147.1	3,537,000	3,144,000	442,200	393,000	190,300	246.0
1031.9	3.84			3,041,000	427,600	380,100	182,400	238.0
992.4	3.83			2,933,000		366,700	176,000	230.0
951.3	3.81			2,824,000		353,000	168,100	222.0
915.2	3.80			2,726,000		340,700	162,000	215.0
879.6	3.79	122.2	2,957,000	2,629,000	369,700	328,600	156,000	208.0
840.0	3.78			2,522,000		315,300	148,500	200.0
801.1	3.77			2,417,000		302,100	141,100	192.0
766.8	3.76			2,322,000		290,300	135,400	185.0
731.3	3.74			2,226,000		278,300	128,300	177.0
695.5	3.73	97.8	2,392,000	2,126,000	299,000	265,700	122,700	170.0
660.0	3.72			2,027,000		253,300	117,100	162.0
626.0	3.70	88.5	2,175,000	1,933,000	271,800	241,600	110,400	155.0
592.6	3.69	84.0	2,070,000	1,840,000	258,700	230,000	103,800	
558.5	3.68	79.3	1,961,000	1,743,000	245,100	217,900	98,550	140.0
523.7	3.66	74.6	1,850,000	1,645,000	231,300	205,600	92,100	133.0
290.0	2.69	55.0	1,825,000	1,622,000	228,100	202,700	132,500	136.5
491.7	3.65	70.2	1,749,000	1,554,000	218,600	194,300	85,800	125.0
274.5	2.68	52.2		0 1,547,000	COLUMN THE REAL PROPERTY.	193,400	125,600	1 1 10 10 10
259.4 $244.5$				$0   1,473,000 \\ 0   1,400,000$		10 mg ac 05 05 05 05	118,700	4 4 54 5
24.6.0	MILOO	3.000	1,000,000					



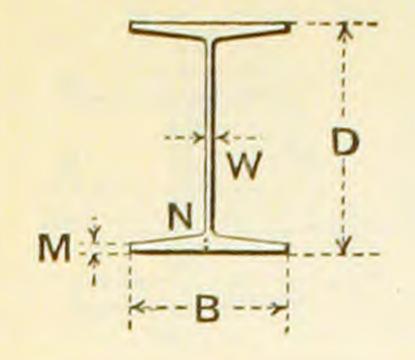
## PROPERTIES OF BETHLEHEM SHAPES

	Weight	Nominal Depth	Width		ICKNE		Area	A	XIS X-	×
Section Number	per Foot, Pounds	of Beam, Inches	of Flange, Inches	Web	Flai	nge	of Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion,	Section Modulus, Inches <sup>3</sup>
		D	В	W	М	N		I	Inches	S
$H^{\frac{1}{1}\frac{0}{2}}$	x 113.0	11.000	12.260	.650	1.057	1.173	33.25	710.8	4.62	129.2
H10	x 112.0	11.250	10.390	.780	1.202	1.298	33.00	699.9	4.60	124.4
$H^{\frac{1}{1}\frac{0}{2}}$	x 107.0	10.875	12.230	.620	.997	1.113	31.45	663.5	4.59	122.0
H10	x 106.5	11.125	10.350	.740	1.139	1.236	31.26	654.7	4.58	117.7
$H^{\frac{1}{1}\frac{0}{2}}$	x 100.0	10.750	12.200	.590	.932	1.048	29.54	613.9	4.56	114.2
H10	x 100.5	11.000	10.310	.700	1.077	1.173	29.53	610.6	4.55	111.0
$H^{\frac{1}{1}\frac{0}{2}}$	x 94.0	10.625	12.170	.560	.867	.983	27.63	565.7	4.52	106.5
H10	x 95.0	10.875	10.280	.670	1.014	1.111	27.91	568.9	4.51	104.6
$H^{\frac{1}{1}\frac{0}{2}}$	x 88.0	10.500	12.140	.530	.807	.923	25.86	522.1	4.49	99.4
H10	x 89.0	10.750	10.240	.630	.952	1.048	26.20	527.2	4.49	98.1
$H^{\frac{1}{1}\frac{0}{2}}$	x 82.0	10.375	12.100	.490	.747	.863	23.98	478.6	4.47	92.2
								486.6		
								447.2		
1								433.2		
								408.9		
1								390.0		
1								371.7		
$H^{\frac{1}{1}\frac{0}{2}}$	x 62.0	10.000	11.990	.380	.557	.673	18.29	350.1	4.38	70.0

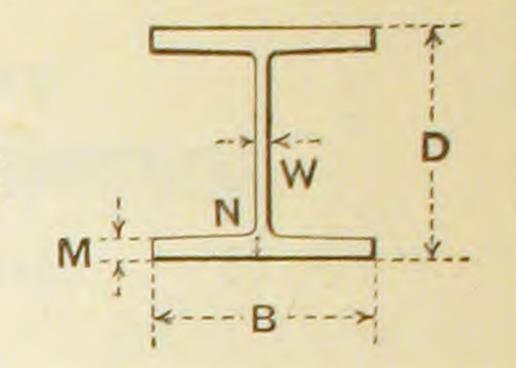
#### BETHLEHEM SHAPES



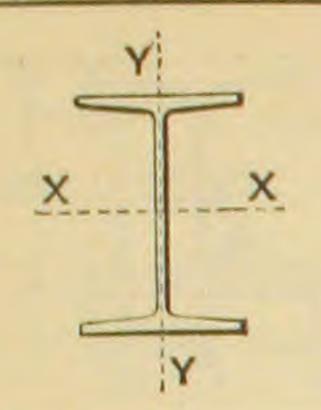
A	(IS Y-Y		COEFFICI STRENG FOOT PO	TH IN	MOMEN RESISTA FOOT P	NCE IN	Maxi- mum	Weight
Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Safe Shear on Web, in Pounds	per Foot, Pounds
		- 5						
334.3	3.17	54.5	1,551,000	1,378,000	193,800	172,300	85,800	113.0
229.9	2.64	44.3	1,493,000	1,327,000	186,600	165,900	105,300	112.0
313.5	3.16	51.3	1,464,000	1,301,000	183,000	162,600	80,950	107.0
215.7	2.63	41.7	1,412,000	1,255,000	176,500	156,900	98,790	106.5
291.5	3.14	47.8	1,371,000	1,218,000	171,300	152,300	76,110	100.0
201.7	2.61	-39.1	1,332,000	1,184,000	166,500	148,000	92,400	100.5
269.7	3.12	44.3	1,278,000	1,136,000	159,800	142,000	71,370	94.0
188.6	2.60	36.7	1,255,000	1,116,000	156,900	139,500	87,440	95.0
249.8	3.11	41.2	1,193,000	1,061,000	149,200	132,600	66,780	88.0
175.2	2.59	34.2	1,177,000	1,046,000	147,100	130,800	81,270	89.0
229.6	3.09	37.9	1,107,000	983,700	138,300	123,000	61,030	82.0
162.0	2.57	31.8	1,099,000	977,100	137,400	122,100	75,230	83.5
149.1	2.56	29.4	1,022,000	908,600	127,800	113,600	69,300	77.5
208.3	3.08	34.5	1,014,000	901,600	126,800	112,700	55,350	75.0
136.5	2.54	27.0	945,900	840,800	118,200	105,100	63,500	72.0
187.8	3.05	31.2	924,800	822,000	115,600	102,800	50,270	68.0
124.2	2.53	24.6	870,300	773,600	108,800	96,700	57,810	66.0
168.7	3.04	28.1	840,200	746,900	105,000	93,360	43,740	62.0



### BETHLEHEM SHAPES

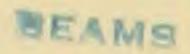


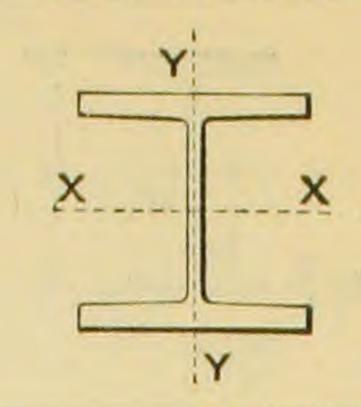
	Weight	Nominal Depth	Width		INCHE		Area	AXIS X-X			
Section Number	FOOT		of Flange, Inches	Web	Flai	nge	of Section, Square Inches	Moment of Inertia, Inches <sup>4</sup>	Radius of Gyra- tion,	Section Modulus Inches <sup>3</sup>	
		D	В	w	M	N		I	Inches	S	
TT10	60 5	10 105	10.040	420	620	796	17 77	225 5	191	66.9	
H10 H10		10.125		.430	.639	.736	1	335.5 300.4	4.34 4.32	66.3	
GIU	x 50.0	10.120	9.040	.300	.445	.807	14.02	277.5	4.50	04.0	
H10	x 49.5	9.875	9.970	.360	.514	.611	14.57	267.2	4.28	54.1	
$H^{-\frac{1}{8}0}$	x 47.5	10.000	8.110	.390	.596	.673	13.90	251.3	4.25	50.3	
G10	x 44.5	10.000	9.000	.320	.385	.747	13.14	246.7	4.33	49.3	
G10	x 41.5	9.910	8.990	.310	.340	.702	12.23	225.8	4.30	45.6	
$H^{-\frac{1}{8}0}$	x 42.5	9.875	8.070	.350	.533	.611	12.49	223.0	4.23	45.2	
$H^{\frac{1}{8}}$	x 38.0	9.750	8.030	.310	.471	.548	11.09	195.6	4.20	40.1	
$H^{\frac{1}{8}}$	x 33.5	9.625	8.000	.280	.408	.486	9.80	169.9	4.16	35.3	
B10	x 28.5	10.190	5.785	.285	.380	.609	8.41	154.1	4.28	30.2	
			5.770						4.24		
			5.750					123.2	4.21	24.6	
B10	x 21.0	9.900	5.740	.240	.235	.464	6.28	108.1	4.15	21.8	
BJ10	x 16.5	10.000	4.000	.240	.240	.397	4.86	77.4	3.99	15.5	
G9	x 43.5	9.120	8.540	350	405	746	12.73	195.4	3.92	42.8	
G9	x 38.5				.345		11.35		3.89	38.2	
G9	x 36.0						10.66	160.5	3.88	35.9	
В9	x 22.0	9.060	5.510	.260	.280	.499	6.51	93.9	3.80	20.7	
B9	x 20.5				.250			86.5	3.77	19.2	



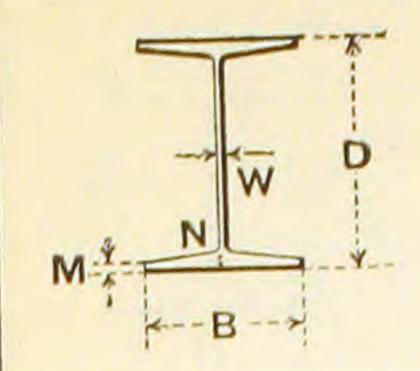
#### BETHLEHEM SHAPES

USED AS COLUMNS

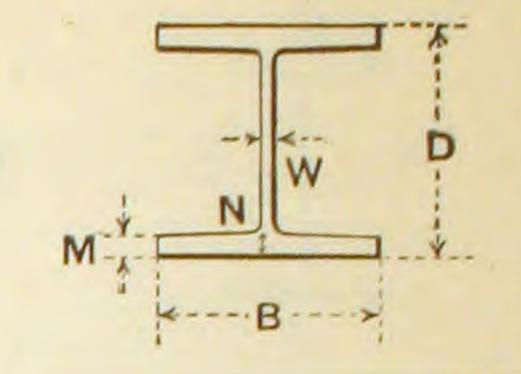




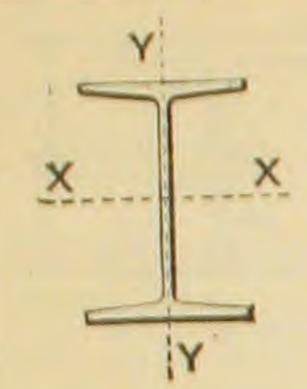
AX	(IS Y-Y		STRENC FOOT P	STH IN	RESISTA FOOT P	NCE IN	Maxi- mum Safe	Weight
Moment of Inertia, Inches <sup>4</sup>	Radius of Gyra- tion Inches	Section Modu- lus, Inches <sup>8</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	per Foot, Pounds
I'	r'	_S'_	C		R	R'		
					00 110	00.050		00 =
112.2	2.51	22.3	795,300	706,900	99,410	88,370	51,780	60.5
100.4	2.50	20.1	721,000	640,900	90,130	80,110	45,230	55.0
66.4	2.13	14.7	658,000	584,900	82,250	73,110	41,500	50.0
89.1	2.47	17.9	649,300	577,100	81,160	72,140	40,240	49.5
54.8	1.99	13.5	603,000	536,000	75,380	67,000	45,230	47.5
58.2	2.10	12.9	592,000	526,300	74,010	65,780	35,000	44.5
52.6	2.07	11.7	546,900	486,100	68,360	60,760	33,200	41.5
48.5	1.97	12.0	542,000	481,800	67,750	60,220	38,760	42.5
42.4	1.96	10.6	481,600	428,100	60,200	53,510	32,390	38.0
36.6	1.93	9.1	423,600	376,500	52,940	47,060	27,600	33.5
14.2	1.30	4.92	362,900	322.600	45,370	40,330	29,500	28.5
12.5	1.28	4.33	328,100	291,600	41,010	36,460	26,900	26.0
10.9	1.25	3.80	295,600	262,800	36,950	32,850	23,600	23.5
9.30	1.22	3.24	262,200	233,000	32,770	29,130	21,900	21.0
3.02	.79	1.51	185,700	165,000	23,210	20,630	21,740	16.5
51.3	2.01	12.0	514,200	457,000	64,270	57,130	37,200	43.5
44.4	1.98	10.4	458,500	407,500	57,310	50,940	31,300	38.5
41.0	1.96	9.67		382,900	53,850	47,870	28,400	36.0
9.42	1.20	3.42	248,600	221,000	31,080	27,630	24,100	22.0
8.54	1.18	3.10	230,700	205,100	28,840	25,630	22,600	20.5



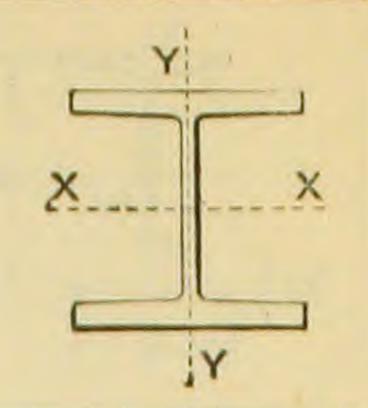
### BETHLEHEM SHAPES



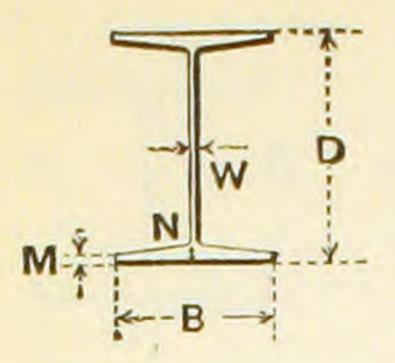
Section Number	Weight per Foot, Pounds	Depth of Beam	Width				Axis x-x			
	per of		Flange, Inches	lange,		nge	of Section, Square Inches	Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modulus, Inches <sup>3</sup>
		D	В	w	М	N		1	r	S
									0.00	01.4
H8	$\times$ 91.0	9.500	8.470			17 17 17 17 17 17	26.77	386.8	3.80	81.4
		9.375					25.33	360.5	3.77	76.9
H8		9.250	8.390				23.91	335.0	3.74	72.4 68.2
H8		9.125	8.360				22.59	311.0	3.71 3.68	63.8
H8		9.000	8.320	.630			21.18	$287.1 \\ 264.0$	3.65	59.5
H8		8.875	8.280	.590			19.79 18.40	241.7	3.62	55.2
H8		8.750	8.240	.550			17.03		3.60	51.0
H8		8.625	8.200	.470			15.66		3.57	46.9
H8 H8		8.375	8.120	.430			14.31		3.54	42.8
H8		8.250					12.96		3.51	38.7
H8	x 39.5	8.125	8.040	.350	.524	.601	11.63	141.0	3.48	34.7
								132.6		
								123.0		
								118.9		
	*		UI II					116.1		
								107.2		
								103.8		
								100.7		
H_8	x 27.0	0 7.875	6.530	.280	.413	.476	7.89	89.7	3.37	22.8
								76.1		
0.		0 8.060								
B8	x 17.	5 8.000	5.250	.250	.210	.418	5.20	57.7	3.33	14.4
		5 8.000								11.2



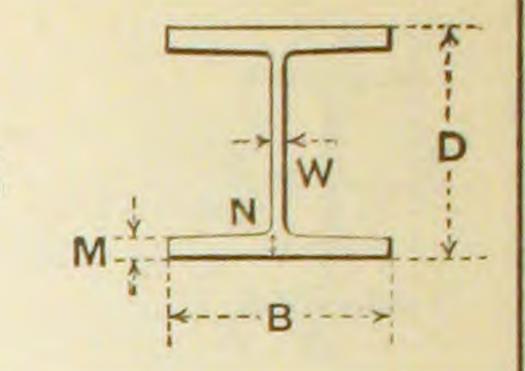
### BETHLEHEM SHAPES



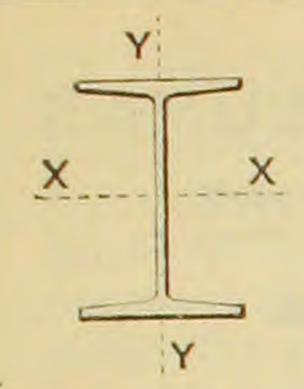
A	XIS Y-Y			ENTS OF STH IN OUNDS		NTS OF NCE IN OUNDS	Maxi- mum Safe	Weight
Moment of Inertia, Inches	Radius of Gyra- tion, Inches	Section Modu- lus, Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	Pounds
I'	r'	S'	C	C'	R	R'		
1051	9.16	29.6	977,200	868,700	122,200	108,600	88,920	91.0
125.1 117.1	2.16	27.8	922,900	820,400	115,400	102,500	83,250	86.0
109.2	2.14	26.0	869,200	772,700	108,700	96,580	77,700	81.5
102.0	2.12	24.4	817,900	727,100	102,200	90,880	73,370	77.0
94.5	2.11	22.7	765,600	680,500	95,700	85,070	68,040	72.0
87.2	2.10	21.1	713,900	634,600	89,240	79,320	62,840	67.5
80.0	2.09	19.4	662,900	589,200	82,860	73,650	57,750	62.5
73.1	2.07	17.8	612,500	544,400	76,560	68,050	52,790	58.0
66.4	2.06	16.3	562,600	500,100	70,330	62,510	47,940	53.0
59.8	2.04	14.7	513,400	456,400	64,170	57,040	43,220	48.5
53.4	2.03	13.2	464,700	413,100	58,090	51,640	38,610	44.0
47.2	2.01	11.7	416,600	370,300	52,070	46,290	33,960	39.5
39.0	1.90	9.72	391,800	348,300	48,980	43,530	29,400	36.5
41.1	2.00	10.3	369,000	328,000	46,120	41,000	28,710	35.0
26.6	1.62	8.07	351,300	312,300	43,920	39,040	33,960	34.5
33.6	1.86	8.39	348,200	309,500	43,530	38,690	26,500	33.0
35.8	1.96	8.95	326,800	290,500	40,850	36,320	28,260	32.0
23.2	1.61	7.07	311,480	276,900	38,940	34,610	28,710	30.5
28.4	1.81	7.10	306,800	272,700	38,350	34,090	25,600	29.5
20.0	1.59	6.11	273,300	243,000	34,170	30,370	24,730	27.0
16.8	1.57	5.18	235,600	209,400	29,450	26,180	20,830	23.5
7.20	1.13	2.73	189,600	168,500	23,700	21,070	23,800	19.0
6.39	9 1.11	2,44	173,200	153,900	21,650	19,240	21,200	17.5
2.73	.80	1.41	134,700	119,700	16,830	14,960	19,920	14.5



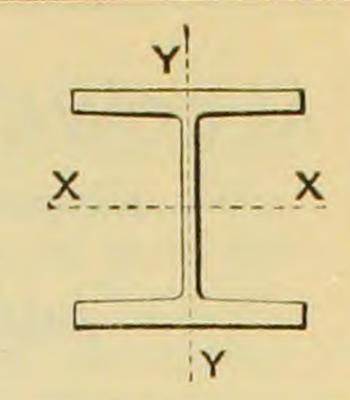
## PROPERTIES OF BETHLEHEM SHAPES



	Weight	Nominal Depth	Width		INCHI		Area	A	XIS X->	
Section Number Foot, Pounds		of Beam, Inches	of Flange, Inches	Web	Fla	nge	Square of Gyra- Mo Inches Inertia, tion, In		Section Modulus Inches <sup>3</sup>	
		D	В	W	М	N		Inches*	Inches	S
2.										
1 1/			10.400	.990	.942	1.037		216.9	2.89	59.7
			10.315							
1 0			10.241							
			10.175							
			10.099							
			10.022							
$H_{10}^{6}$	x 46.0	6.356	9.944	.534	.488	.582	13.54	97.4	2.68	30.6
H6	x 40.5	6.750	6.220	.470	.721	.779	11.87	90.5	2.76	26.8
H 1 6	x 40.0	6.216	9.875	.465	.418	.512	11.71	82.3	2.65	26.5
H6	x 37.0	6.625	6.180	.430	.659	.716	10.83	80.9	2.73	24.4
			6.140		1					
H6	x 30.0	6.375	6.100	.350	.534	.591	8.77	62.8	2.68	19.7
H6	x 26.5	6.250	6.060	.310	.471	.529	7.76	54.4	2.65	17.4
H6	x 23.0	6.125	6.020	.270	.409	.466	6.76	46.4	2.62	15.2
BS6	x 20.5	6.188	6.060	.300	.330	.388	6.06	41.5	2.62	13.4
H6	x 20.0	6.000	6.000	.250	.346	.404	5.89	39.1	2.58	13.0
BS6	x 18.0	6.094	6.030	.270	.285	.343	5.33	35.8	2.59	11.7
BS6	x 15.5	6.000	6.000	.240	.240	.298	4.61	30.3	2.56	10.1
BJ6	x 11.0	6.000	3.330	.230	.230	.359	3.25	19.3	2.44	6.43



### BETHLEHEM SHAPES



A	(IS Y-Y		STRENG FOOT P	TH IN	RESISTA	NTS OF ANCE IN OUNDS	Maxi- mum Safe	Weight
Moment of Inertia, Inches <sup>4</sup>	Radius of Gyra- tion Inches	Inches <sup>3</sup>	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	For Fiber Stress of 18,000 Lbs. per Sq. In.	For Fiber Stress of 16,000 Lbs. per Sq. In.	Shear on Web, in Pounds	Foot, Pounds
I'	r'	_S'_	· C		R	R'		
182.0	2.65	35.0	716,600	637,000	89,580	79,630	86,310	88.0
162.0	2.62	31.4	648,500	576,500	81,060	72,060	77,060	80.0
145.0	2.60	28.3	589,500	524,000	73,690	65,500	69,270	73.0
130.9	2.58	25.7	539,800	479,800	67,470	59,980	62,590	67.0
114.9	2.55	22.7	482,100	428,500	60,260	53,560	55,110	60.0
99.3	2.52	19.8	424,700	377,500	53,090	47,190	47,820	53.0
84.1	2.49	16.9	367,700	326,900	45,970	40,860	40,730	46.0
29.6	1.58	9.52	321,900	286,200	40,240	35,770	38,070	40.5
71.1	2.46	14.4	317,600	282,300	39,700	35,290	34,690	40.0
26.6	1.57	8.60	293,000	260,400	36,620	32,550	34,190	37.0
23.6	1.55	7.70	264.500	235,100	33,070	29,390	30,420	33.5
20.8	1.54	6.82	236,500	210,300	29,570	26,280	26,780	30.0
18.1	1.53	5.96	209,000	185,700	26,120	23,220	23,250	26.5
15.4	1.51	5.12	181,800	161,600	22,730	20,200	19,850	23.0
12.8	1.45	4.23	161,000	143,100	20,139	17,890	22,250	20.5
13.0	1.49	4.34	156,500	139,100	19,560	17,390	17,700	20.0
11.0	1.43	3.64	141,000	125,300	17,620	15,660	19,580	18.0
9.1	9 1.41	3.06	121,100	107,600	15,140	13,450	16,630	15.5
1.6	4 .71	.98	3 77,160	68,590	9,645	8,574	15,850	11.0

### ALLOWABLE UNIT STRESSES FOR COLUMNS

AMERICAN INSTITUTE OF STEEL CONSTRUCTION, 1923

Main, and Short Secondary Members: ratios  $\frac{l}{r}$  from 0 to 120.

Allowable stress in Pounds per Square Inch:

15,000 for ratios  $\frac{l}{r}$  from 0 to 60.

$$\frac{18,000}{l^2} \text{ for ratios } \frac{l}{r} \text{ from 60 to 120.}$$

$$1 + \frac{l}{18,000 \text{ r}^2}$$

Ratio l/r  60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80	Allowable Stress, Pounds per Square Inch  15,000 14,916 14,832 14,748 14,663 14,578 14,493 14,407 14,321 14,235 14,148 14,062 13,975 13,888 13,801 13,714 13,627 13,540 13,453 13,366 13,279	Difference  A  84  84  84  85  85  85  86  86  87  87  87  87  87  87  87  87	Ratio l r  80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100	Allowable Stress, Pounds per Square Inch  13,279 13,192 13,105 13,018 12,931 12,844 12,758 12,672 12,585 12,500 12,414 12,328 12,243 12,158 12,073 11,989 11,905 11,821 11,737 11,654 11,571	Difference  Δ  87  87  87  87  87  88  88  88  88	Ratio l r  100 101 102 103 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119 120	Allowable Stress, Pounds per Square Inch  11,571 11,489 11,407 11,325 11,244 11,163 11,082 11,002 10,922 10,843 10,764 10,686 10,688 10,530 10,453 10,376 10,376 10,376 10,376 10,300 10,224 10,149 10,074 10,000	Difference  Δ  82 82 82 81 81 81 80 80 79 79 78 78 78 77 76 76 76 75 75 74
79	13,366	87	99	11,654	83	119	10,074	75

The tables of Allowable Unit Stresses for Columns give the allowable stress corresponding to each integral value of l/r, together with the difference, A, between each pair of such allowable stresses standing adjacent to each other in the table. As the stresses decrease with an increase in t/r, all the differences are negative. To find the allowable stress for any intermediate value of l/r, look up the value of the stress for the integral part of l/r, and the difference,  $\triangle$ , between this stress and that corresponding to the next higher value of 1/r. Corresponding to this difference there will be found in the Interpolation Table the values of .10  $\triangle$  to .90  $\triangle$  corresponding to the .1 increments in l/r. If l/r is given to hundredths, one tenth the increment for ten times the number of hundredths and the increment for the number of tenths may be added together. As this increment is negative, subtract the number so obtained from the stress for the integral part of l/r. The result is the allowable stress for the given t/r. The computed difference should be rounded up to the units place before subtracting.

### INTERPOLATION TABLE FOR DIFFERENCES

				INC	REME	NTS			
Δ	.10 A	.20 ∆	,30 Δ	,40 A	,50 Δ	,60 Δ	,70 △	.80 △	.90 Δ
87	8.7	17.4	26.1	34.8	43.5	52,2	60.9	69.6	78.3
86	8.6	17.2	25.8	34.4	43.0	51.6	60.2	68.8	77.4
85	8.5	17.0	25.5	34.0	42.5	51.0	59.5	68.0	76.5
84	8.4	16.8	25.2	33.6	42.0	50,4	58.8	67.2	75.6
88	8.8	16.6	24.9	33,2	41.5	49.8	58.1	66.4	74.7
82	8.2	16.4	24.6	32.8	41.0	49,2	57.4	65.6	73.8
81	8.1	16.2	24.3	32.4	40.5	48.6	56.7	64.8	72.9
80	8.0	16.0	24.0	32.0	40.0	48.0	56.0	64.0	72.0
79	7.9	15.8	23.7	31.6	39.5	47.4	55.3	63.2	71.1
78	7.8	15.6	23.4	31.2	39.0	46.8	54.6	62,4	70.2
77	7.7	15.4	28.1	30.8	38.5	46.2	53.9	61.6	69.3
76	7.6	15.2	22.8	30.4	38.0	45.6	53.2	60.8	68.4
75	7.5	15.0	22.5	30.0	37.5	45.0	52.5	60.0	67.5
74	7.4	14.8	22.2	29.6	37.0	44.4	51.8	59.2	66.6

It should be noted that the calculation could be based on the stress for l/r = 108 in the above problem, in which case  $\Delta l/r = -.63$ , and the increments of stress are all positive, which simplifies calculations somewhat.

### ALLOWABLE UNIT STRESSES FOR COLUMNS

AMERICAN INSTITUTE OF STEEL CONSTRUCTION, 1923

Secondary Members Only: ratios  $\frac{l}{r}$  from 120 to 200.

Allowable unit stress in Pounds per Square Inch:

$$\frac{18,000}{l^2} \text{ for ratios } \frac{l}{r} \text{ from 120 to 200.}$$

$$1 + \frac{18,000 \text{ r}^2}{18,000 \text{ r}^2}$$

No column or strut is permitted whose  $\frac{l}{r}$  is greater than 200.

Ratio l	Allowable Stress, Pounds per Square Inch	Differ- ence	Ratio l	Allowable Stress, Pounds per Square Inch	Differ- ence	Ratio 1/r	Allowable Stress, Pounds per Square Inch	Differ- ence
120	10,000	74	146	8,241	61	173	6,760	49
121	9,926	73	147	8,180	60	174	6,711	49
122	9,853	73	148	8,120	60	175	6,663	48
123	9,780	72	149	8,060	60	176	6,616	47
124	9,708	72	150	8,000	59	177	6,568	48
125	9,636	72	151	7,941	59	178	6,521	47
126	9,564	71	152	7,882	58	179	6,475	46
127	9,493	70	153	7,824	57	180	6,429	46
128	9,423	70	154	7,767	57	181	6,383	46
129	9,353	69	155	7,710	57	182	6,338	45
130	9,284	69	156	7,653	56	183	6,293	45
131	9,215	69	157	7,597	56	184	6,248	45
132	9,146	68	158	7,541	55	185	6,204	44
133	9,078	67	159	7,486	55	186	6,160	44
134	9,011	67	160	7,431	54	187	6,117	43
135	8,944	66	161	7,377	54	188	6,074	43
136	8,878	66	162	7,323	53	189	6,031	43
137	8,812	66	163	7,270	53	190	5,989	42
138	8,746	65	164	7,217	53	191	5,947	42
139	8,681	64	165	7,164	52	192	5,906	41
140	8,617	64	166	7,112	51	193	5,864	42
141	8,553	63	167	7,061	52	194	5,824	40
142	8,490	63	168	7,009	50	195	5,783	41
143	8,427	63	169	6,959	51	196	5,743	40
144	8,364	62	170	6,908	50	197	5,703	40
145	8,302	61	171	6,858	49	198	5,664	39
146	8,241		172	6,809	49	199	5,625	39
			173	6,760		200	5,586	39

### INTERPOLATION TABLE FOR DIFFERENCES A

					INC	REMENT	s			
4	Δ -	,10Д	.20△	.30 △	.40△	,50△	,60△	,70△	.80 △	.90Δ
	74	7.4	14.8	22,2	29.6	37.0	44,4	51.8	59,2	66.6
	73	7.3	14.6	21.9	29,2	36.5	43.8	51.1	58.4	65.7
	72	7.2	14.4	21.6	28.8	36.0	43.2	50.4	57.6	64.8
	71	7.1	14.2	21.3	28.4	35.5	42.6	49.7	56.8	63.9
	70	7.0	14.0	21,0	28,0	35.0	42.0	49.0	56.0	63.0
	69	6.9	13.8	20.7	27,6	34.5	41,4	48,3	55.2	62.1
	68	6.8	13.6	20,4	27.2	34.0	40.8	47.6	54.4	61.2
	67	6.7	13,4	20.1	26.8	33.5	40.2	46.9	53,6	60,3
	66	6.6	13.2	19.8	26,4	33.0	39.6	46.2	52,8	59.4
	65	6.5	13.0	19.5	26.0	32.5	39.0	45.5	52.0	58.5
	64	6.4	12.8	19.2	25.6	32.0	38.4	44.8	51.2	57.6
	63	6.3	12.6	18.9	25,2	31.5	37.8	44.1	50,4	56.7
	62	6,2	12.4	18,6	24.8	31.0	37.2	43,4	49,6	55.8
	61	6.1	12.2	18.3	24.4	30.5	36.6	42.7	48,8	54.9
	60	6.0	12.0	18.0	24.0	30.0	36.0	42.0	48.0	54.0
	59	5.9	11.8	17.7	23.6	29.5	35.4	41.3	47.2	53.1
	58	5.8	11.6	17,4	23,2	29.0	34.8	40.6	46.4	52,2
	57	5.7	11,4	17,1	22,8	28.5	34.2	39.9	45.6	51,3
	56	5.6	11,2	16.8	22,4	28,0	33.6	39.2	44.8	50.4
	55	5.5	11.0	16,5	22,0	27.5	33.0	38.5	44.0	49.5
	54	5,4	10.8	16,2	21,6	27.0	32,4	37.8	43.2	48.6
	53	5.3	10.6	15,9	21,2	26.5	31,8	37.1	42,4	47.7
	52	5.2	10.4	15.6	20.8	26.0	31,2	36.4	41.6	46.8
	51	5.1	10.2	15,3	20,4	25.5	30.6	35.7	40.8	45.9
	50	5.0	10,0	15.0	20,0	25.0	30.0	35.0	40.0	45,0
	49	4.9	9.8	14.7	19,6	24.5	29.4	34.3	39.2	44.1
	48	4.8	9.6	14.4	19,2	24.0	28.8	33,6	38.4	43,2
	47	4.7	9,4	14,1	18.8	23,5	28.2	32,9	37.6	42.3
	46	4,6	9,2	13.8	18.4	23.0	27,6	32.2	36.8	41.4
	45	4.5	9.0	13,5	18.0	22,5	27.0	31.5	36.0	40,5
	44	4.4	8,8	13.2	17.6	22,0	26.4	30.8	35.2	39,6
	43	4.3	8.6	12.9	17,2	21,5	25.8	30,1	34.4	38.7
	42	4,2	8.4	12,6	16.8	21,0	25.2	29,4	33.6	37.8
	41	4,1	8,2	12,3	16,4	20,5	24.6	28.7	32.8	36.9
	40	4.0	8,0	12,0	16,0	20.0	24.0	28.0	32.0	36.0
	39	3,9	7.8	11.7	15.6	19.5	23.4	27.3	31.2	35.1

## MOST ECONOMICAL BETHLEHEM SHAPE TO USE AS A BEAM FOR A REQUIRED SECTION MODULUS

Sect		Section	Weight per Foot,	Secti		Section Number	Weight per Foot,
From	То	Tunno	Pounds	From	То		Pounds
10	6.4	ВЈ 6	v 11	201.8	222.1	B28 2	85
4.8	11.2		x 14½	222.2	246.9	B28	.91
6.5	15.5	1000	x 16½	247.0	265.1	B28	x 97
11.3	20.2	2011	x 18½	265.2	284.7	B28	x 104
15.6	21.8		x 21	284.8	314.8	B30	x 110
20.3 $21.9$	24.6		$\times 23\frac{1}{2}$	314.9	330.8	B30 :	x 115
21.9	31.2		2 x 25	330.9	351.3	B30 :	x 121
31.3	35.6		2 x 28	351.4	395.1	B33	x 125
35.7	42.5		1 x 30	395.2	422.3	B33 :	x 135
42.6	47.8		1 x 33	422.4	449.4	B33	x 143
47.9	55.1		3 x 35	449.5	503.4	B36	x 147
55.2	59.7		5 x 38½	503.5	530.4	B36	x 155
59.8	65.8		3 x 40	530.5	561.1	B36	x 164
65.9	73.8		3 x 45	561.2	595.0	B36	x 173
73.9	85.2	B 18	8 x 47	595.1	659.9	B36	x 190
85.3	89.2	B 18	8 x 49	660.0	672.4	G33	x 200
89.3	94.3	B 18	8 x 52	672.5	707.3	G33	x 210
94.4	113.3	B 25	2 x 54½	707.4	741.4	G33	x 220
113.4	124.7	B 2	2 x 58	741.5	778.0	G33	x 230
124.8	135.9	B 2	2 x 62½	778.1	833.9	G36	x 230
136.0	148.1	B 2	$2 \times 67\frac{1}{2}$	834.0	872.0		x 240
148.2	163.7	B 2	4 x 70	872.1	911.2		x 250
163.8	175.7	B 2	4 x 73½	911.3	949.5		x 260
175.8	188.2	B 2	4 x 79½	949.6	1030.8		x 280
188.3	201.7	B 2	6 x 81	1030.9	1103.6	G36	x 300

Beams to be secured against yielding sideways.

### BETHLEHEM STEEL COMPANY

BETHLEHEM, PA.

#### PARTIAL LIST OF PRODUCTS

STRUCTURAL STEEL SHAPES: Bethlehem Beams, Joists and Stanchions; Rolled Girder Beams, Rolled Columns; Standard Beams, Channels and Angles; Standard and Special T and Z Bars; Plain and Fabricated; Crane Rails; Rolled Steel Slabs for Column Bases.

SHIPBUILDING SHAPES: Ship Channels, Bulb Angles, and Hatch Sections.

CAR BUILDING SHAPES: Beams, Channels, Angles, Bulb Angles, Z Bars, Center and Side Sill Sections, Belt Rail, Door Spreader, and Side Stake Sections.

PLATES: Universal and Sheared; Circular (Heads), in all grades for all purposes; Miscellaneous Pressed Work.

PILING: Lackawanna Steel Sheet Piling.

BRIDGES AND FABRICATED BUILDINGS: Designers, Builders, Fabricators and Erectors of all types of Bridges and Steel Structures. Buckle Plates.

RAILROAD TURNTABLES: Bethlehem Twin-Span Turntables; Balanced and Continuous Turntables.

FLANGED AND DISHED BOILER HEADS, SPECIAL FLANGED PRODUCTS.

AGRICULTURAL STEEL AND SPECIALTIES: Standard and Special Shapes.

AUXILIARY LOCOMOTIVES.

BARS AND BANDS: Muck Bar, Refined, Double Refined, Chain, Stay Bolt, Special Stay Bolt, Horseshoe and Engine Bolt Iron; Bessemer, Open Hearth, Electric and Alloy Steel; Concrete Reinforcing Bars.

BILLETS, BLOOMS, SLABS, SKELP AND SHEET BARS.

BOILER TUBES: Lap Welded; Charcoal Iron, and Steel.

BOLTS, NUTS, RIVETS, SPIKES, POLE LINE MATERIAL.

CARS: STEEL AND COMPOSITE FREIGHT, STEEL PASSENGER, MINE AND INDUSTRIAL.

CAR WHEELS: Rolled Steel.

CASTINGS: Steel, Iron, Brass and Bronze; Stainless Clad; Centrifugal.

COAL: Gas and Steaming.

ENGINES: Blowing, Producer Gas, and Gas.

FERRO-MANGANESE, SPIEGELEISEN, COKE AND COKE BY-PRODUCTS.

FORGINGS: Drop, Upsetter, Hammered and Hydraulically Pressed; All sizes and types; Forged Shafts.

GEARS AND PINIONS: Cut and Cast; Bridge Operating Machinery.

INDUSTRIAL AND MINE TRACK WORK, STEEL MINE AND INDUSTRIAL TIES.

INGOT MOULDS: All sizes.

MACHINERY: Hydraulic Machinery and Equipment; Special Machinery of all types and designs.

OIL BURNING SYSTEMS.

OIL REFINERY EQUIPMENT.

PIG IRON: Standard Grades, Special Grades, Mayari, Silvery Mayari.

PIPE AND TUBULAR GOODS: Lap and Butt-welded, Pipe, Casing and Tubing.

PULVERIZERS FOR COAL AND OTHER MATERIALS.

RAILS AND ACCESSORIES, FROGS AND SWITCHES. BETHCO RAIL ANCHORS.

ROLLED STEEL BLANKS FOR GEARS, PINIONS, FLY WHEELS, ETC.

ROLLS: Carbon and Alloy Steel.

SHEET AND TIN MILL PRODUCTS: ROLL ROOFING, RIDGE ROLL. SPECIAL STEEL FOR AUTOMOBILE FORGINGS AND MACHINED PARTS.

STEEL AXLES: For Passenger and Freight Cars, Engine and Tender Trucks; Driving;

Motor; Electric and Mine Car; etc.

Tool Steel, Carbon and Alloy: Bethlehem Special High-Speed Tool Steel; Non-shrinkable; Rock and Mine Drill Steel; Special Tool Steel; Small Tools, etc.

WIRE RODS, WIRE NAILS, WIRE, Woven Field and Poultry Fencing. Steel Fence Posts.

#### PLANTS AT

Bethlehem, Pa.; Lebanon, Pa.; Coatesville, Pa.; Johnstown, Pa.; Steelton, Pa.; Lackawanna, N. Y.; Sparrows Point, Md.; Wilmington, Del.

### BETHLEHEM STEEL COMPANY

### General Offices

### BETHLEHEM, PENNSYLVANIA

#### District Offices

New York	Cunard Building
Boston A	tlantic National Bank Building
Philadelphia	Widener Building
Baltimore	Continental Building
Washington, D. C	Wilkins Building
Atlanta	Healey Building
Pittsburgh	Oliver Building
Buffalo	Marine Trust Building
Cleveland	
Cincinnati	Union Trust Building
Detroit	New Penobscot Building
Chicago	People's Gas Building
St. Louis	Arcade Building
Houston	Post Dispatch Building

### Pacific Coast Distributor

### PACIFIC COAST STEEL CORPORATION

San Francisco	Matson	Building
Los Angeles	. Pacific Finance	Building
Seattle	L. C. Smith	Building
Portland, Ore	. American Bank	Building
Honolulu	Castle and Cook	Building

#### Export Distributor

BETHLEHEM STEEL EXPORT CORPORATION 25 BROADWAY, NEW YORK CITY

